

## Highcliffe Drive

was based on the new intersection layout which includes an extended southbound left-turn lane, and the conversion of the northbound and southbound right-turn lanes to shared through/right-turn lanes.
Section 4.3 describes the reconstruction in further detail.

### 3.3 Traffic Data

To determine the existing conditions at the study intersection, turning movement counts were undertaken on Wednesday, July 31, 2019 by Spectrum Traffic Data Inc. (Spectrum) from 7:00 a.m. to 10:00 a.m. and from 4:00 p.m. to 7:00 p.m. The traffic count data is detailed in Appendix D, and the 2019 existing traffic volumes are illustrated in Figure 5.

The Region was consulted for turning movement counts, however the latest counts they had were from 2014, which does not comply with the two-year time frame outlined in the Region's "Transportation Mobility Plan Guidelines".

Peak hour factors (PHF) associated with the weekday a.m. and p.m. peak hours were calculated for the study intersections based on the existing traffic volumes. Table 2 outlines the PHFs as calculated and applied to the model for the study intersections.

Table 2: Traffic Data Summary

| Intersection | Peak Hour | Peak Hour Factor |
| :---: | :---: | :---: |
| Bathurst Street and Flamingo <br> Road/Worth Boulevard | Weekday AM (8:30-9:30) | 0.92 |
|  | Weekday PM (17:30-18:30) | 0.98 |
| Flamingo Road and the Site <br> Access | Weekday AM (8:15-9:15) | 0.84 |
|  | Weekday PM (17:00-18:00) | 0.94 |

### 3.4 Traffic Modelling

The assessment of the study intersections is based on the method outlined in the "Highway Capacity Manual, 2000" using Synchro 9 modelling software. The intersections are assessed using a Level of Service metric with ranges from "A" to "F". A Level of Service "A" to "C" would typically be measured during off-peak hours when lesser volumes are on the roadways. Levels of Service "D" through "F" would typically be measured in the commuter peak hours when greater vehicle volumes cause longer travel times. The Level of Service definitions for stop-controlled and signalized intersections are included in Appendix E.
$9^{\text {th }}$ percentile queue lengths were derived from the average of five runs in Sim Traffic with a seeding interval of 10 minutes and a recording interval of 60 minutes.

Per the Region's Transportation Mobility Plan Guidelines, an ideal (base) saturation flow rate of 2000 vehicles per hour per lane was assumed in the Synchro modelling, and the peak hour factors were based on the existing traffic counts as noted above.

The signal timing plan for the intersection of Bathurst Street and Flamingo Road/Worth Boulevard was obtained from Viva staff and was used in the analysis of the existing conditions. The signal timing plan has been included in Appendix $\mathbf{F}$ for reference.

As summarized in Section 3.5, the eastbound left-turn movement experiences a volume-to-capacity ratio in excess of 1.00 . This is a result of Synchro considering the westbound right-turns as opposing

Reduced lane width on both sides of Flamingo Rd.
Cars can no longer turn right onto Bathurst on red if car ahead is waiting to go through the intersection, causing further traffic congestion.


