



STRUCTURAL CONDITION ASSESSMENT FOR 89 NASHVILLE ROAD, KLEINBERG, ONTARIO



Prepared for STATEVIEW HOMES

November, 2019 Project ID: AS19-145



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November 11, 2019

Stateview Homes 410 Chrislea Road, Unit 16 Woodbridge, Ontario L4L 8B5 Email:

Attention: Mr. Daniel Ciccone, CFO

Re: 89 Nashville Road, Kleinberg, Ontario Structural Assessment Project ID: AS19-145

Wynspec Engineering (Wynspec) was authorized by Stateview Homes. to undertake a structural (visual) assessment of the dwelling unit located at 89 Nashville Road, Kleinberg, ON in accordance with our proposal.

The purpose of our review was to determine the present condition of the structural elements forming the dwelling unit for the above captioned site after an incident of reported vandalism. In conjunction with the building structure, a review of the foundation walls, roof deck structure, roofing system, and exterior cladding elements was also undertaken as part of the investigation.

Our findings and methodology are summarized in detail in the attached report. The findings of the condition assessment were used to evaluate the capability of typical structural elements within the existing building structure, given their present condition, together with our conclusions regarding the factors contributing to the development of each type of deficiency noted to be present as it relates to the overall structural integrity of the dwelling unit The primary purpose of our condition survey was to assess and document the existing condition and to give assurance that the unit can be temporarily relocated and returned to the existing site.

Regardless of your plans for this property, any activities in the building—whether construction, renovation or demolition— should be performed with extreme care as several areas of framing have a potential for collapse if subjected to potentially even minor dynamic forces.

We would be pleased to review the contents of this report with you should you so request.

Should you have any questions please contact our office.

Yours truly, Wynspec Engineering

Izzie Katzenberg, P.Eng., B.D.S. Senior Structural Engineer



1.0 BACKGROUND

Wynspec Engineering was contacted by Mr. Daniel Ciccone of Stateview Homes regarding vandalism that occurred on July 26, 2019, at 89 Nashville Road, in Kleinberg, Ontario. A police report was filed (Occurrence Number 19-254750-99) by Constable Petrillo badge # 2359 and Constable Mohammed badge # 2372. The vandalism reportedly caused localized damage to the floors and walls. We were asked to assess the physical condition of the structure, identify apparent structural damages, deficiencies and severity, provide our opinions and recommendations based on our investigation results. We understand that Stateview Homes has proposed to lift and relocate the dwelling unit and at a later date reinstate the dwelling unit to its original address. In addition to providing our recommendations in response to the observed condition of specific structural components. Wynspec undertook a survey of the residential dwelling unit at 89 Nashville Road, Kleinberg ON. It is our understanding that the City of Vaughan's Cultural Heritage Resource Assessment Inventory List has identified the property at 89 Nashville Road as item 852. The original construction materials forming the dwelling unit at the above captioned site has been greatly reduced and modified over many years from its original structure. The original veneer on the dwelling unit have been removed long ago.

The property reviewed has been has been vacant for a number of years. The intent of the survey was to obtain selected information on the current physical condition of the structural members, identify deficiencies that would affect the structural integrity and durability of the structure. From the building condition assessment, the dwelling unit may contain asbestos and possibly other hazardous materials. While these materials have to be handled and removed under provincial regulations it should not be viewed as an overwhelming or deciding factor because the cost of hazardous materials remediation is a constant value whether the building is demolished or retained.

2.0 DESCRIPTION

The 1½ storey, appears to be a mid 19th century Georgian /Classic Revival style house with a saltbox addition to the rear tail and with several other modifications and alterations that are concealed with metal siding and insulbrick. Insulbrick was popular as an exterior siding between the 1940's and early 1950's. Windows, doors and sills throughout have all been replaced, sills and shutters. The original portion of the dwelling unit was built prior to any Building Codes in Canada. These types of homes were not designed or built to meet modern building practices, as such, this building will not control the movement of air, heat, water vapour or sound. The residential dwelling unit located at 89 Nashville Road, Kleinberg, ON comprises of field stone, brick and block foundation walls, with light wood framing and timber framing. The exterior wall assembly is believed to consist of metal siding, insulbrick wide simple plank boards, plaster lath, and stud walls, however the wall cavity space was not directly observed. The interior walls and ceiling on the upper floor are of a lathe and plaster as well as gypsum board construction with a painted finish. The test openings revealed vermiculite insulation or transit. The residential dwelling unit appears to have numerous modifications carried out over the years as well as evidence of extensive mold, moisture and decay within the attic space. The main floor and upper floor levels of the home are finished. The main floor has three entranceways, hallways, family room, living room, dinning room, bathroom and kitchen at the rear tail of the saltbox addition.

The upper floor has a bathroom, three bedrooms and a bathroom. The basement area is partially finished with an addition to the south section of the dwelling unit forming a salt box style to the tail of the dwelling unit. Knob and tube-wire insulators at several locations are an intriguing remnant of earlier wiring installations. The basement also has a cellar and a laundry room. It is apparent that the property has been abandoned for several years. The roof assembly throughout have plain soffit boards and appear to be deteriorated at several locations. Deterioration was observed at the north west corner of the basement foundation walls. The field stone repairs at the southeast corner of the foundation appears to be spalling, delaminated and scaling of stone and cementitious repair materials.









WEST ELEVATION

SOUTH ELEVATION

3.0 PURPOSE AND METHODS OF THE INVESTIGATION

The objective of this investigation included the assessment of the existing structural members, such as the roof, bearing walls, floor framing, foundations for decisions on future action, specifically to enable a review to be made of appropriate remedial repair options or decommissioning of the existing dwelling units.

The field work consisted of the following:

3.1 Methodology

- .1 Visual review of the structural components;
- .2 Visual review the roof, exterior walls, interior walls floors and foundation walls to check for visible signs of structural defects; .3
- Sounding Survey
- .4 Check floor slope, identifying areas with potential drainage or ponding concerns;
- .5 Test openings (walls and ceilings) to check the as constructed conditions
- .6 Measuring the cross-sectional areas of structural framing members to confirm if any significant deterioration has occurred;

Visual Survey

A visual review of the general condition of the structural framing, roofing assemblies and foundation walls; Visual walk-through review and survey of the dwelling unit (surface, framing, and roofing assembly) to identify visually obvious signs of deterioration and to detect areas of possible leakage, and other possible forms of deterioration; and,

A visual survey was carried out to determine for the following conditions:

- \triangleright Evidence of leakage.
- Locations of active water leakage; ⊳
- ⊳ Cracking;
- Deterioration of the foundation wall assemblies ≻
- ۶ Deterioration of timber framing;
- ≻ Peeling paint; and,
- \triangleright A visual review of walls and floors on all levels.

Sounding and Probing Survey



One of the most commonly used techniques for detecting deterioration is to hit the surface of a member with a hammer or other objects. Based on the sound quality or surface condition, we can identify areas of concern for further investigation. Deteriorated areas typically have a hollow or dull sound that may indicate internal decay. A pick hammer is commonly used and recommended for use in timber structures because it allows us to combine the use of sound and the pick end to probe the element.

Probing with a moderately pointed tool, locates decay near the wood surface as indicated by excessive softness or a lack of resistance to probe penetration and the breakage pattern of the splinters. A brash, or brittle break indicates decayed wood, whereas a splintered break indicates sound wood. Although probing is a simple inspection method, experience is required to interpret results. Care must be taken to differentiate between decay and water-softened wood, which may be sound but somewhat softer than dry wood. Probes can also be used to assess the depth of splits and checks in timber members

Test Openings

Several test openings were carried out on the interior and exterior walls as well as the ceilings to expose the condition of the timber frame members for further review.

Conclusions regarding the factors contributing to the development of the deficiencies now present in this structure, together with recommendations are detailed at the end of this report. No calculations were performed to confirm the adequacy of the original design. We did not undertake any analysis of the structural framing assembly performance, energy efficiency, adequacy of the design.

Photographs were taken to illustrate the various deficiencies noted on the interior and exterior of the dwelling unit and are included in this report.

4.0 STRUCTURAL OBSERVATIONS AND RESULTS OF FIELDWORK

A visual examination of the home was performed on the months of October 2019 and November 2019. The dwelling unit was found to be in total disrepair with vast amount of debris on the floors mainly on the ground floor. The debris was mainly composed of damaged and loose ceilings, partitions, wall and floor coverings and broken fixtures. The windows were broken on all floors. The front, rear and side entrances were missing or damaged with debris lying on the main floor which was found to be unsafe for inspection. The exterior of the house has visible signs of structural damage and water damage. It appears roof leaks have occurred in the past or may continue to occur based on the existing condition of the asphalt shingles and sheathing concealing the roofing members and limited staining of the insulation and wood framing in the attic space. The cladding materials were all a minimum of 8" above the finished grade except the north and west walls of the house. There are large areas of missing or deteriorated shingles as well as damage to the plywood sheathing and simple wide plank boards concealing the roofing members. This condition has provided rainwater access to the interior of the attic space degrading and decaying the roof framing and upper and main floor levels. There also appears to be evidence of soot on the roof framing members and connections.

The basement area was inspected first located under the main floor of the Family Room and Hallway. Numerous floor joists have been notched or split. Several deteriorated floor joists had been replaced or doubled up for structural purposes. The field stone foundations were examined and found to be extensively deteriorated and bowing outwards at several locations. The concrete block foundations were examined where accessible and were found to be in fair condition. Some evidence of water staining was present along the perimeter foundation walls. The summer beams supporting the timber framing forming the main floor were found to be damp, split and degraded and irreparably damaged in some areas and were supported with temporary post shores and concrete block pilasters.

Field Stone and Concrete Block Foundations

The rubble field stone foundation is in poor condition. There is visible extensive deterioration. The exterior walls are constructed of ruble field stone. The varied coursing pattern, uneven in some areas, indicates a rapid construction. The mortar joints are heavily deteriorated. Most of the foundation walls will likely have to be replaced. The concrete block foundation walls forming the newer additions were observed and found to be in fair to poor condition. Evidence of water penetration through the walls was present at some locations. There is evidence of structural distress observed on the concrete blocks forming the foundation walls. Some blocks were found to be cracked or spalled along the exposed exterior of the foundation forming the addition. Step cracks on the south west addition indicating some form of differential movement between what's left from the original structure and with the updated structure. The south west



section forming the addition is susceptible to collapse depending on the continued movement associated with the lack of bracing and end connections

The poured concrete floor slab which has encountered some deterioration, especially at the foot of the staircase, and is also evident on the floor and sill plate. This is likely due to standing water at a recent point in time. The presence of moisture retention in the concrete is evident from dampness to the touch, and from visible leaks from the foundation walls.



Evidence of extensive concrete deterioration and spalling concrete Evidence of spalling and concrete delamination concealing the concealing the original loose field stone foundations at the south west corner. the foundation walls appear to be bowing outwards.



Extensive structural damage to the exposed original field stone. Extensive structural damage to the exposed original field stone. Several stones were found to be loose, as shown. Several stones were found to be concealed and painted with cementitious repair materials, as shown.





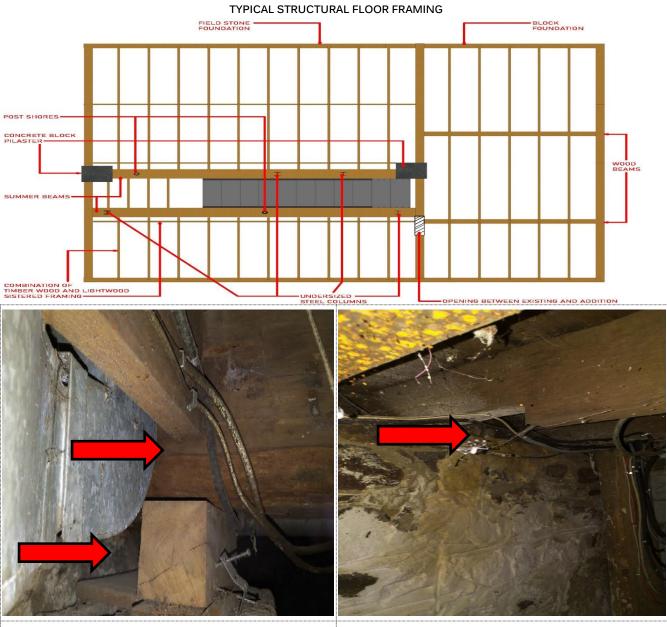
Evidence of step cracking in the mortar joints in the block Extensive staining and vegetation on the concrete block foundation wall at the south west elevation.

Floor Framing

Construction of the floor joist framing throughout the home is light wood framing and timber framing and appears to be in poor condition where accessible. Several floor joists in the basement were found to be split and notched at load bearing points. The floor joists that have been cut or notched at the ends may not be able to sustain normally accepted construction or residential level floor loads. The flooring throughout the home has been badly affected by falling damp and ingress of rainwater over a prolonged period of time. The extent of the degradation was hard to gauge because of several overlays concealing the original flooring. The overlays have also degraded at numerous locations in the presence of damp, mold and rot. The overlay was generally resistant to forceful probing in localized areas, allowing access to minimal areas of the floor. In some sections of the floor, the original flooring had degraded and disintegrated. The underside of the second floor was examined through test openings and found to exhibit extensive mold spores. The spacing of the floor joist were taken and ranged between 400mm and 450mm OC which do not currently conform to the Ontario Building Code requirements. No load sharing system was observed on the underside of the floors between joists. No use of cross bridging, strapping or any combination of these as a means of supporting loaded floors were observed.

Several original post and beams have been replaced with undersized lumber. Temporary post shores, undersized steel columns and concrete block pilasters are in place supporting the main floor level. In general, there is evidence of extensive deterioration sagging floors, beams deflecting and structural elements that show signs of structural distress.





Several framing members were found to be split and degradedSeveral framing members are deteriorated and have been notchedand were supported on structurally unsound concrete blockout, split and have also warped. Alterations can weaken members.pilasters with 4x4 blocks.This type of modification can severely weaken the joists.





Heavy timber framing is split and rotting in several locations. The Several framing members have been notched out and have split staining or discoloration of the wood members indicates that it has been subjected to water and potentially has a high moisture content suitable to support decay.



Temporary post shores and 6x6 timber have been placed to support the timber floor framing and summer beams. The concrete block pilaster appears to have moved with the slab on grade. The summer beam was found to be degraded and rot in several areas.





Undersized steel columns placed on the slab on grade below the summer beam. It appears that no footings were created for the steel columns. It appears that the slab on grade is heaving creating an upward punching shear force through the timber wood summer beam.

Exterior Walls

Exterior Wall Assembly – Metal siding, insulbrick and locally removed and replaced simple wide plank boards, and plaster lath. The metal siding and insulbrick were removed from the exterior face of the wall assembly and a visual inspection from grade was carried out and the wood framing was found to be locally replaced or in poor condition.

Although some degree of deterioration is more pronounced in localized areas, in general, the following defects were observed:

- Wood rot
- Damaged wood
- Peeled paint
- Missing or loose boards
- Holes in cladding

While the structure does not appear to have settled for the exception of the localized section of the south west corner, it is probable that the siding was replaced due to deterioration. In several places, window sill's have deteriorated, which suggests that excessive moisture has been trapped inside and drying of the wood has been insignificant and deterioration has occurred at different levels.

The wood framing concealed with the metal siding and insulbrick appears to be unidentified hardwoods. The wood framing appears to be deteriorating on the upper floor walls and some pieces appear to be experiencing rot. The ground floor appears to be deflecting independently from the original structure. Our test openings revealed no bridging, cross bracing or end connections that have been notched or temporally supported on post shores and concrete block pilasters at several locations. At corner locations it was observed that the top plates did not overlap or tie into the additions or modifications to the home. Solid blocking was missing between several joists spans. No blocking was found at test opening between studs. The inconsistency of wood framing and connections from mid 1800's to post 1970's construction was found throughout all of our test openings and appears to be affecting the overall structural integrity and performance of the home. The deterioration is expected to increase at an accelerated rate if left unattended and action is not taken to reinstate the effectiveness and integrity of the structural framing forming the exterior walls and load bearing walls.





The exterior framing and simple wide plank boards that have been locally replaced are currently degraded and show evidence of wood rot. These framing members are concealed with insulbrick and metal siding.

The exterior framing and simple wide plank boards that have been locally replaced are currently locally exposed to the elements, however it appears that it was concealed with insulbrick and metal siding.



The South West section was exposed and reviewed. The exterior The North East section was exposed and reviewed. The exterior framing framing and simple wide plank boards that have been locally and simple wide plank boards that have been locally replaced. Several boards and framing members were found to split and exhibit extensive wood rot.





The corner location was exposed and found to contain diagonal The upper portion of the timber bracing found to be disengaged bracing. The bracing members found to be moist and damp with and deteriorated, as shown in the photo above. evidence of mold. The upper portion of the timber bracing was found to be deteriorated and split.

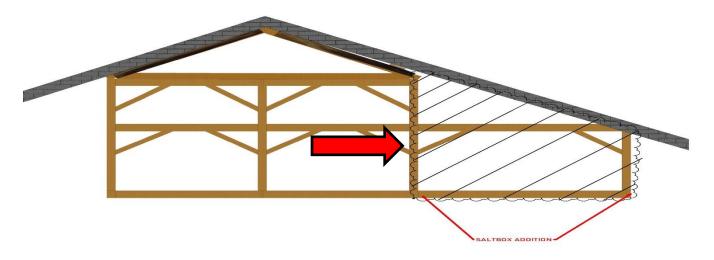


Several simple plank boards and timber framing were found to The exterior wall and interior wall between the original structure exhibit extensive mold growth. Several exterior boards between the and the salt box addition have been vandalized. The walls are original dwelling unit and the salt box tail have been locally concealed with plaster lath. replaced.



Saltbox Addition

The saltbox alteration to the rear tail of the dwelling unit was constructed at a later date. The saltbox foundation wall is constructed of concrete blocks. The concrete block foundation wall appears to be in fair condition however, the south west corner appears to be settling and heaving. There are step cracks in the block foundation wall. This may be evidence of differential movement in the block foundations. There are some leaks into the block foundation wall as well as vegetation on the exterior face of the block foundations. The framing appears to be light wood construction. The floor framing below this area appears consist of two timber support beams with light wood framed joists. The interior walls have been concealed with plaster lath or gypsum. The exterior walls have been concealed with metal siding and insulbrick.



Rear saltbox framing addition identified in the hatched areas. The exterior walls forming the saltbox are constructed of timber framing and light wood framing.



Timber wood beams supporting the wood light framing below the saltbox addition. Note, evidence of water staining along the upper portion of the foundation walls.



Rear Addition

The wood framed addition forms as a mudroom. This structure is bearing on the wood decking at that rear of the dwelling unit. The structure does appear to have settled. The wood framing below the addition is structurally unsound and on the verge of collapse. A portion of the addition also being supported by concrete blocks that are placed above grade. The wood joists forming the decking and support for the addition has decayed and there is extensive deterioration.



Rear addition, concealed with metal siding and roof assembly Rear addition bearing on the wood decking and concrete blocks concealed with asphalt shingles. placed above grade.



There is evidence of wood rot and wood decay on several joists The concrete blocks are temporary supports for the rear addition. Supporting the rear addition. The rear addition has settled and is structurally unsafe and in an unsound condition.



Roof Framing

Roof and Drainage System. The asphalt shingled roof was not accessible for close inspection. The roof shingles are not original and are in poor condition. Water stains were found in several locations on the second floor, which would suggest leaks in the roof. At these locations it is possible that the surrounding roof joists have deteriorated due to moisture.

The rainwater disposal system consists of aluminum gutters and rainwater leaders that are not original to the building. The existing drainage system is in poor condition. Based on the findings of this review, it is our opinion that the gable roof framing was found to be severely damaged with obvious signs of warping in plane along with some of the framing being split. At several critical elements of the roof framing a disturbing structural fault was observed between connections between pre1900's construction and post 1970's construction. Some framing seemed to be experiencing rot from areas exposed from prolonged exposure to falling damp, and adverse affects of weather. Several sections of the roof framing have been altered and construction practices in these locations is poor and do not meet current Ontario Building Code requirements. Top edges of light wood framing have been adversely affected to a level that could not be determined from the type of inspection conducted. Several timber framing was missing between the old and new structure. The roofing insulation appears to contain hazardous materials. A designated substance report should be addressed prior to decommissioning the structure from the existing site. The majority of the roofing in the residential dwelling unit is in general state of decay, with numerous holes and missing sections. This degradation has led to a significant number of the faults in the light wood framing and damage observed.

When viewed from the outside of the dwelling unit, obvious sag is evident in the roof framing on the north east elevation. This may be an original condition, or it may be associated with the crushing and splitting of the roof framing. Extreme care should be exercised in entering this part of the dwelling unit until a more thorough examination of the condition of the roof framing can take place.

During our review, we noted that the roof has been re-roofed and the original wood structure supported by wood framing have been locally replaced. An asphalt shingle roof provides protection over the home. The soffits were vented and the roof had a number of roof vents. The condition of the roof framing was found to be generally in a poor condition. The roof framing is concealed with typical plaster lath materials. Several test openings were carried out and access to the attic space was made available at the time of our inspections. The roof framing was found to be deteriorated and in a general state of decay with numerous holes and missing sections. The degradation and alterations has led to significant number of the faults in the light wood framing, particularly in the vicinity of their structural supports. Top edges of light wood framing have been adversely affected to a level that could not be determined from the type of inspection conducted.

Attic venting was found not in accordance with the Ontario Building Code requirements. Inadequate attic venting may translate to apparent water leaks by means of condensation and premature deterioration of the roof framing members. As such, some of the existing exhaust ducts do not vent directly to the exterior of the roof as required be the OBC. Considering the insulation (a common element) was found to be less than the specified R-value in the OBC and in some cases missing along the top plate of exterior walls.

While reviewing the roof framing we also noted that brick chimney stack is deteriorated with missing mortar joints and spalled with loose brick at several locations.





There is extensive wood deterioration forming the simple plank Several timber roof frames were found to have extensive decay board sheathing and timber wood trusses. The timber framing and mold, as shown in the above photograph. wound to be wet and deteriorated at locations made visible.



Timber wood simple plank boards and timber roof assembly foundSeveral timber plank boards were found to exhibit extensiveto exhibit deterioration and in a general state of decay.deterioration as well as mold.





The peak of the roof assembly was found to be moist and damp Several timber roof members were found to be split, and warped with extensive mold growth. at several areas made accessible.

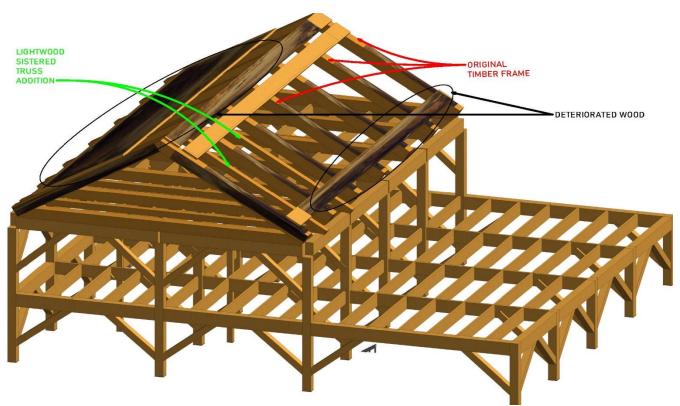


There is an overlay of shingles. The shingles and plywood There are obvious sags evident in the roof framing on the north east sheathing where made accessible were found to be deteriorated. elevation.





The wood soffits appear to have water stains and some wood The metal siding concealing the wood framing for the roof has deterioration, as shown in the photograph. No vents were provided in the separated leaving the wood elements exposed to the elements.



Detail Identifying Deteriorated Roof Framing Assembly



Masonry - Chimney

The building has a brick chimney between the original footprint and the saltbox addition to the rear tail of the dwelling unit. It is likely that the original heating of the home was likely a fireplace in the main house. Although only reviewed from the ground, the brick chimneys appear to have many defective mortar joints, as well as broken and damaged bricks. The flashing appears to have not been correctly installed.



There are several spalled and deteriorated bricks including loose The metal flashing appears to have been installed incorrectly. The mortar joints. The precast coping stone above the bricks was also found to be delaminated and cracked. The metal flashing appears to have been installed incorrectly. The lower portion of the chimney also shows evidence of brick spalls and loose mortar joints



5.0 LIMITATIONS

The structural members were visually reviewed for signs of deterioration and/or damage. No analysis of the structure was performed at this time. The information presented in this report is based on direct visual observation made by personnel with Wynspec and in some instances as noted within the report on information provided by others. Recommendations contained within our report reflect our informed opinion based on the information gathered during our investigation. The findings cannot be extended to components of the building or portions of the site that were not reviewed or that were concealed or unavailable for direct observation at the time of our visit. There is a possibility for additional deficiencies being present in the building which have not been identified during our visit, given the limited nature of this review.

No legal survey, soil test, detailed structural engineering investigation, or quantity survey compilation have been made. No responsibility, therefore, is assumed concerning these matters, or for any failure to carry out those technical or engineering procedures required to discover any inherent or hidden condition of this property since such investigation work was not included in the terms of reference governing this study.

The conclusions and recommendations detailed in this report are based upon the information available at the time of preparation of the report. No investigative method eliminates the possibility of obtaining imprecise or incomplete information. Professional judgement was exercised in gathering and analyzing the information obtained and in the formulation of our conclusions and recommendations. The recommendations are not intended to be utilized as a detailed specification for any remedial work that may be required. WYNSPEC accepts no responsibility for interpretation of our recommendations, or actions taken based on them without our consultation and supervision.

Information provided by WYNSPEC is intended for the exclusive use of **the client**. WYNSPEC will not provide results or information to any party other than the client, unless the client, in writing, requests that information be provided to a third party or unless disclosure by WYNSPEC is required by law. Any use by a third party, of reports or documents authored by WYNSPEC, or any reliance by a third party, or decisions made by a third party, on the findings described in reports or documents authored by WYNSPEC, is the sole responsibility of such third parties. WYNSPEC accepts no responsibility for damages suffered by any third party as a result of decisions made or work carried out based on reports or documents authored by WYNSPEC.

WYNSPEC makes no representations concerning the legal and medical significance of our findings. With respect to regulatory compliance requirements, regulations change from time to time, and interpretation of their meaning and intent may also change. WYNSPEC accepts no responsibility for any legal interpretation of the Regulations, or the consequent financial effect on transactions, property values, or requirements for follow-up actions and costs.

The liability of WYNSPEC or its staff is limited to the fees paid or actual damages incurred by the client, whichever is less. WYNSPEC is not responsible for consequential or indirect damages. All claims by the client shall be deemed relinquished if not made within two years after last date of services provided.

The client expressly agrees that it has entered into this agreement with WYNSPEC, both on its own behalf and as agent on behalf of its employees and principals.

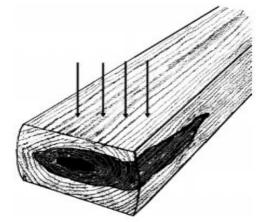
The client expressly agrees that WYNSPEC's employees and principals shall have no personal liability to the client in respect of a claim, whether in contract, or tort, or in any other cause of action in law. Accordingly, the client expressly agrees that it will bring no proceedings and will take no action in any court of law against any of WYNSPEC's employees or principals.

The following photographs detail the observations made during our investigation, documenting visual cues in the form of cracking, evidence of water penetration structural deficiencies, hazardous materials and degradation as shown below: Our recommendations are based upon limited observations and our experience with similar buildings built in the mid to late 1800's in Ontario. Due to the lack any drawing, we used various means to develop framing cross sections which may not be accurate and may not represent actual conditions. The data presented in the report are for exclusive use of Statview Homes and shall not be used in any way for design and construction.



MECHANISMS OF DETERIORATION AND MOISTURE

In general, deterioration of timber and light wood framing structures is a result of exposure from the combined presence of water penetration and moisture. Degradation occurs as a result of moisture and leaks through cracks and defects in the walls and roofing assemblies. If this process is left unchecked, and allowed to continue, the structural capacity of the component can be significantly reduced as a result of the deteriorated timber framing and light wood framing.



Because of interior deterioration the outer shell can collapse when the timber is stressed in bearing.

6.0 DISCUSSION AND CONCLUSIONS

6.1 FIELD STONE AND CONCRETE BLOCK FOUNDATIONS

The block foundation walls were constructed years ago and form part of the pre 1970's addition. The block foundation walls at locations made accessible were visually reviewed. The south west corner of the addition was found to exhibit step cracking and heaving of the block. The cementitious repair materials at this location was found to be extensively delaminated and unsound. This location appears to be structurally unsound. The original fieldstone foundation walls forming the basement of the mid to late 1800's dwelling unit appear to be structurally unsound and have been repaired over the years with cementitious repair materials and masonry brick. It was also observed that the basement flooded commonly during raining periods. The external field stone walls are generally bowing outwards. The stonework is suffering from erosion of the mortar joints close to defective gutters or flashings. The stonework has been pointed with cement mortar, and this has resulted in water becoming entrapped in the stone and subsequent frost damage There is evidence of laterally and differential movement in the foundation walls indicating that this is creating unsafe condition structurally.

6.2 FLOOR FRAMING

The findings of this review suggest the primary concern for the floor slabs is the effectiveness of the existing floor joists and support beams. The degradation and alterations on the underside of the floor slabs is visually alarming. The floor framing is an important element of the building structural stability to establish a stiff diaphragm in the absence of any diagonal bracing. The flooring and floor framing on the main floor and upper level is in a variable condition with some parts being in an extremely poor condition. Several temporary post shores, block pilasters and undersized makeshift beams have been placed to support the ground floor framing.

Cracking and staining of plaster lath were identified at several locations on the upper lower levels. There are no cracks visible externally. Elsewhere cracking of plaster lath is evident within the new addition and hallways most likely resulting from rainwater penetration via the failed flashing details. Floors appear to have an excessive bounce at several locations. The gaps between boards is, in some instances, excessive and reduces the capacity of the floors to function fully as plan diaphragms to restrain the external walls.

6.3 EXTERIOR WALLS

The exterior walls have been significantly modified. The original veneer has been removed and replaced with metal siding and at an earlier date replaced with insulbrick. The exterior framing has been modified with the addition to the rear tail to the original structure that was found to be constructed poorly and also found to be degraded, split and stained with evidence of water penetration. Our test



openings revealed cross-bracing at the corner location however, no bridging or strapping was placed. No lintels or proper headers above openings or window locations were present during our review. The south modification to the structure shows movement cracking and apparent deflection.

6.4 SALTBOX ADDITION

The timber wood beams below the rear tail forming the saltbox addition show signs of sag, likely due to long-term creep wood under the load of the terracotta floor finishes above within the kitchen area. The beams also exhibit ineffective load-sharing between the foundation walls. We did see some visible signs of wood deterioration and split members to the main structural framing system forming the ground floor and exterior walls. The observed damage showed visible signs of structural distress in the form of excessive deformations, movements, cracking, spalling, sagging floors. There were obvious signs of damage to the supporting timber framing beams and columns.

6.5 REAR ADDITION

There is little structural value in the rear addition. Large areas of failed and deficient framing would have to be completely rebuilt. The remaining framing in some areas is unlikely to have sufficient strength for current code-specified occupancy loads. The structure, including the temporary block foundations on grade and the decayed wood framed decking, would not likely be structurally adequate to support its self weight or any additional loads necessary to reach a reasonably level of serviceability. Given the current state of the existing structure overall, little, if any, framing is salvageable

6.6 ROOF FRAMING

During our review, we noted that the roof framing has been re-roofed and the original wood structure supported by wood timber framing has also been locally replaced. A new light wood framed roof was added to the rear tail end of the home forming the salt box addition and small addition forming a rear addition. An asphalt shingle roof provides protection over the home. The asphalt shingles concealing the roofing assembly was found to be deteriorated and missing in several sections. The condition of the roof framing was found to be generally in a poor condition. The underside of the roof framing is concealed with typical plaster lath materials. Several test openings were carried out and limited access to the attic space was made available at the time of our inspections. The roof framing was found to be deteriorated and in a general state of decay with numerous holes and missing sections. The degradation and alterations have led to significant number of the faults in the light wood framing, as well as to the original timber framing particularly in the vicinity of their structural supports. Signs of serious wet-rot decay were noted in the timber. Top edges of light wood framing have been adversely affected to a level that could not be determined from the type of inspection conducted. The roofing insulation also appears to contain hazardous materials. A designated substance report should be addressed prior to decommissioning the structure from the existing site.

No attic venting was found. The OBC requires a venting area 1/300 of the total heating ceiling area, of which 25% is required from the roof and soffit vents. Inadequate attic venting may translate to apparent water leaks by means of condensation and premature deterioration of the roof framing members. Several sections of the roof framing have been altered and construction practices in these locations is poor and do not meet current Ontario Building Code requirements. The east addition was observed and from the outside of the dwelling unit, obvious sag is evident in the roof framing. This may be an original condition, or it may be associated with the crushing and splitting of the roof framing.

6.7 MASONRY- CHIMNEY

The chimney stack to the rear roof has a pronounced, historical, lean inwards towards number 99. This lean is not an immediate cause for concern, as the stack does not extend significantly above the ridge line and is restrained laterally, in the direction of the lean, by the rear roof structure. Some raking out and repointing is likely to be required during general repairs to the flashings.

6.8 HISTORICAL & PHYSICAL ANALYSIS OF THE BUILDING

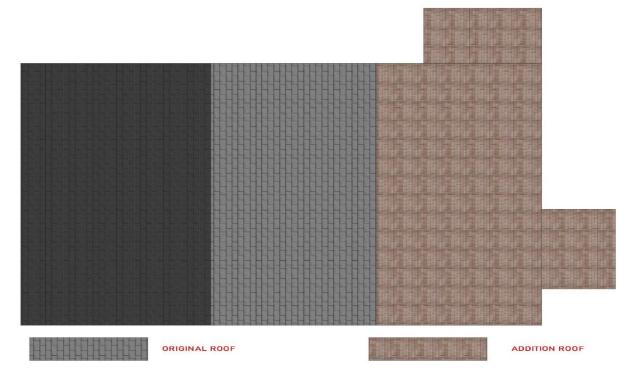
The dwelling unit at 89 Nashville Road has changed significantly based on our site observations and test openings, the existing structure retains a high degree of inconsistencies from pre1900's to post 1970's. The photo to the left below shows the existing dwelling unit captioned in the 1970's and the photo below to the right is the existing dwelling unit located at the above captioned site.





Photographic image of 89 Nashville road taken in the (1970's). Photographic image of 89 Nashville Road in (2016). Detail from aerial photo from the Vaughan Archives. Note the small addition to the rear of the dwelling unit.

Character and context of modifications carried out to 89 Nashville Road over the years as identified below.



6.9 Key Findings

Our test openings at the time of our review revealed that several timber framing was found to exhibit extensive decay and deterioration. Several sections of the roof framing have been altered and construction practices in these locations is poor and do not meet current



Ontario Building Code requirements. The east addition was observed and from the outside of the dwelling unit, obvious sag is evident in the roof framing. This may be an original condition, or it may be associated with the crushing and splitting of the roof framing.

In analysing the dwelling unit, and forming an opinion if the building is structurally sound at this time and code compliance our comments are as follows:

- .1 The building in its present condition is in poor condition, abandoned partially deteriorated, degraded and neglected.
- .2 The saltbox and small rear addition appears to be deflecting independently from the original structure.
- .3 The structural framing members in the roof, walls and floor joists are badly degraded that we would condemn the structure. The structural loads associated with its original use are far in excess of the loads that it will experience with any proposed relocation of the structure.
- .4 The original veneer has been completely replaced with metal siding as well as insulbrick
- .5 Several modifications that were poorly constructed.
- .6 The original framing members within the dwelling unit have been locally replaced.
- .7 Mould spores and hazardous materials were found throughout the dwelling unit. A designated substance report should be carried out.

The dwelling unit on the site has been identified as having heritage value, however based on our review and findings the original structure forming the original dwelling design has been considerably reduced. The building has suffered from moisture, decay, and cyclic maintenance and do present evidence of significant structural distress. The building has been modified and altered with several poorly constructed additions that are all concealed with plaster lath, drywall finishes and metal siding for the exterior walls.

At the time of our visit we also noted that there's no functioning electrical or mechanical components throughout the dwelling unit. With no heating or cooling sources in the dwelling space as well as areas exposed from prolonged exposure to falling damp, and adverse affects of weather will facilitate condensation into the dwelling space and create a dew point on the roof framing and structural support members and will promote further deterioration and decay.

6.10 SUMMARY

We understand that the City of Vaughan has deemed the dwelling unit a Property of Heritage Value. We also understand that if the structure would have to be demolished would require a consent from the Council and Heritage Committee. Our findings have revealed that several critical elements and disturbing structural faults were observed between connections pre1900's construction and post 1970's construction. Some framing seemed to be experiencing rot from areas exposed from prolonged exposure to falling damp, and adverse affects of weather. Several sections of the framing have been altered and construction practices in these locations are poor and do not meet current Ontario Building Code requirements. Mold spores and hazardous materials were also found throughout the dwelling unit. The exiting building facade and structural members have been either modified or reduced from its original state, as observed during our review.

The existing condition of the building is not characteristic of its type or technical feature that would make it potentially aesthetically significant. The original veneer has been removed long ago. We also understand that the dwelling unit has been extensively disturbed with the addition to the south section of the dwelling unit forming the saltbox tail and placed above concrete block foundations along with several additions made to the structure. The small rear addition is supported by temporary on grade concrete blocks and wood decking that has extensive wood deterioration and is structurally unsound. As the proposed development would involve relocation of the dwelling unit or total demolition, Councils Guidelines for demolition requires an applicant to reach a conclusion regarding the heritage significance of the property based on a Heritage Impact Assessment carried out by a Professional Heritage Consultant. This assessment was solely carried out to determine the structural condition of the dwelling unit located at 89 Nashville Road, Kleinberg ON. The first question from a structural point of view is whether there are any structural hazards or can the construction be refurbished or, in extreme cases, needs to be replaced. The choice of action must be based on an assessment of the status of the structure carried out by a Professional Engineer. The assessment is typically based on a mapping of damage to the bearing frame. The mapping of damage needs to be accurate to optimize both the safety level and the best solution for repair methods if possible. This should also include a visual mapping of damage, such as spalling, cracking, delaminations, deformations and other physical influence the foundations and framing structure. The initial review would also require that similar members that show obvious degrees of degradation be identified within the structure.



Based on our experience, visual assessment and test openings we strongly believe that the original structure located at 89 Nashville Road, Kleinberg ON has been structurally modified with poorly constructed alterations and has been structurally compromised at this time. Therefore, we are of the opinion that retention of the property could not be made suitable for continued adapted use however, could be redeveloped for other purposes, subject to obtaining the necessary approval. Given the current state of the existing structure overall, little, if any, framing is salvageable. The remaining framing in some areas is unlikely to have sufficient strength for current codespecified occupancy loads. We are of the opinion that a complete demolition of the existing dwelling unit be carried out with new construction, there is little structural value in the existing dwelling structure, particularly the newer saltbox addition and smaller addition to the rear of the dwelling unit.

In our professional opinion, the damages and alterations due to their structural deficits and poor condition should not be considered worthy of retention for heritage reasons. The dwelling unit is unsound, structurally compromised and hazardous as it stands, and does not have a viable future use. Access to the dwelling unit should be strictly limited until the framing is made safe through shoring, repair or demolition.

At this time, given the structural deficiencies described throughout the report, we don't believe we can give assurance that moving the dwelling unit away and returning it will not result in any further damages or collapse. Furthermore, given the condition of the existing structure, and the potential cost involved for the construction process necessary to move the dwelling unit away and returning it as well as the rehabilitation or retrofit of the structure in such a scenario would result in substantial costs versus the cost to construct a completely new structure of similar magnitude.

We trust that this meets your current needs. If you have any questions or if we may be of further assistance please contact the undersigned.

Yours truly, Wynspec Engineering.

Vinh Dang, B.A.Sc. Building Restoration Specialist

Izzie Katzenberg, P.Eng., B.D.S. Senior Structural Engineer

