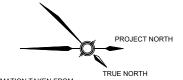
RECEIVED

By Christine Vigneault at 10:40 am, Jun 16, 2023

*Revised by Zoning to reflect proper height measurement.

LOT AREA: 761.81 SQ.M. EXISTING HOUSE FOOTPRINT: 175.28 SQ.M. COVERED DECK: 38.91 SQ.M.



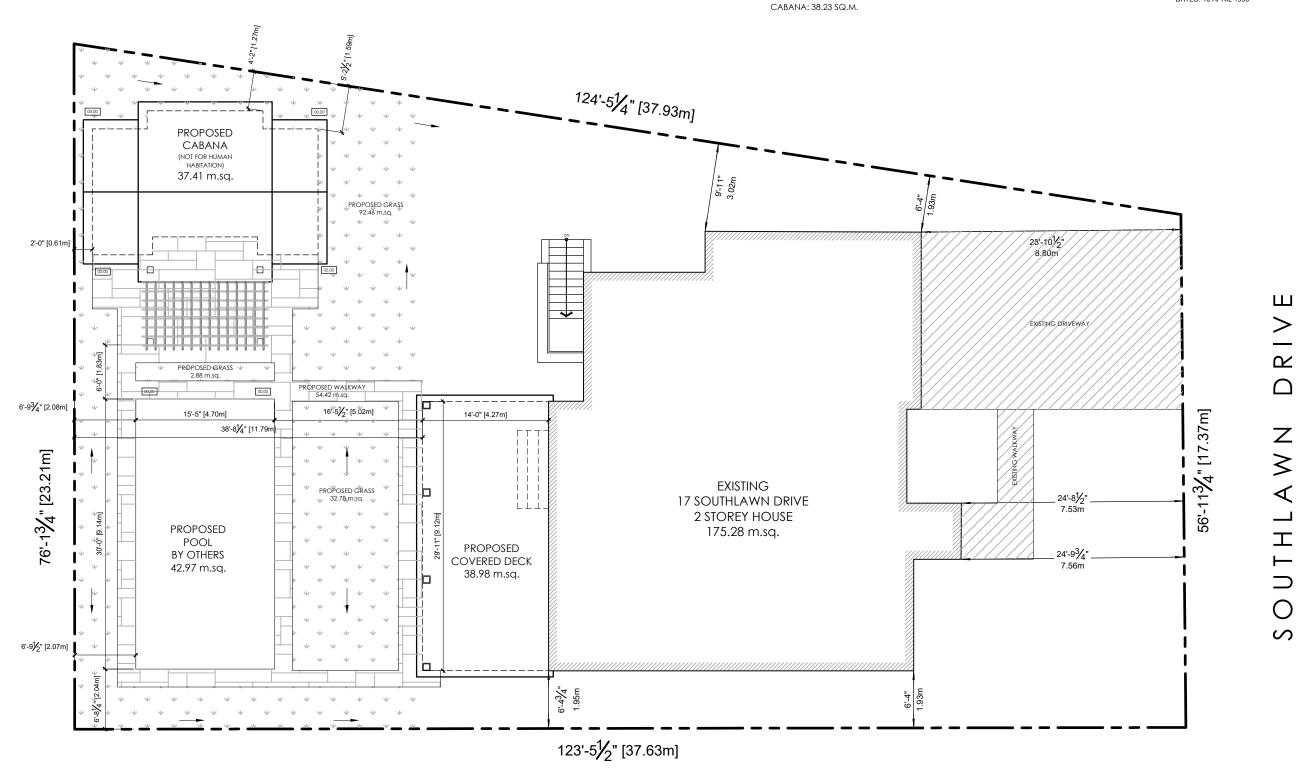
SITE PLAN INFORMATION TAKEN FROM: LOT 2 PLAN 65M - 2987 IN THE TOWN OF VAUGHAN IN THE REGIONAL MUNICIPALITY OF YORK BY: RUDY MAK SURVEYING LTD. DATED: 10 APRIL 1996

<u> </u>	
<u> </u>	OII E PLAN
	SOUTHLAWN
	RESIDENCE
рву	17 SOUTHLAWN DRIVE, WOODBRIDGE, ONT., L4H 1A1

			ISSUED BY
			REVISION/SUBMISSION
			DATE
			No.

GORAL DESIGN T: 647.505.9632





STRUCTURAL NOTES AND SPECIFICATIONS
THESE NOTES ARE TO BE FULLY READ AND UNDERSTOOD IN CONJUNCTION WITH THE DESIGN/CONSTRUCTION PERMIT DRAWINGS.

IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO READ AND FULLY UNDERSTAND THE REQUIREMENTS OF THE PROPOSED WORK. THE CONTRACTOR SHALL CONTACT THE DESIGNER AND/OR ENGINEER. IF THEY HAVE QUESTIONS PERTAINING TO THE WORK PRIOR TO COMMENCING THE

- CONTRACTOR IS RESPONSIBLE FOR PROVIDING SHORING AND/OR TEMPORARY WORKS DURING CONSTRUCTION FOR THE SAFE INSTALLATION OF ALL CONSTRUCTION MATERIALS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONTROL, DIRECTION AND SUPERVISION OF THE CONSTRUCTION METHODS, MEANS TECHNIQUES, SEQUENCES OR PROCEDURES FOR ALL SHORING
- CONTRACTOR SHALL BE RESPONSIBLE FOR THE ACTUAL CONSTRUCTION OF THE WORK AND BE RESPONSIBLE FOR THE CONTROL DIRECTION AND SUPERVISION OF THE CONSTRUCTION METHODS, MEANS, TECHNIQUES, SEQUENCES OR PROCEDURES FOR ALL CONSTRUCTION OF THE WORK.

 THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE HEALTH
- AND CONSTRUCTION SAFETY LEGISLATION AT THE PLACE OF THE
- PRIOR TO THE COMMENCEMENT OF NEW WORKS, THE CONTRACTOR IS RESPONSIBLE TO ENSURE THAT THE EXISTING STRUCTURE IS INTACT AND FREE OF DEFECTS SUCH AS, BUT NOT LIMITED TO, CRACKS, SPALLING, ROT, DEFLECTIONS, DEFORMATIONS AND SETTI EMENTS IT IS THE DUTY OF THE CONTRACTOR TO OPEN AREAS OF EXISTING TO INSPECT THE UNDERLYING STRUCTURE WHERE IT IS THE CONTRACTOR IS TO COMMUNICATE AREAS OF CONCERN TO THE ENGINEER IMMEDIATELY.
- WHERE THE CONTRACTOR IS REQUIRED TO VERIFY SITE CONDITIONS, THIS SHALL MEAN THAT THE CONTRACTOR SHALL EXPOSE THE EXISTING CONDITION AND REPORT THEIR FINDING TO THE ENGINEER
- THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING THE BUILDING DEPARTMENT FOR REQUIRED INSPECTION (REFER TO THE BUILDING PERMIT FOR INSPECTION REQUIREMENTS.) THE DESIGNER AND/OR ENGINEER WILL NOT BE RESPONSIBLE OR HELD LIABLE, IN PART OR IN WHOLE, FOR DESIGN ERRORS RELATED TO THE WORK IF WE ARE NOT RETAINED TO INSPECT THE WORK DURING CONSTRUCTION, IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CONTACT THE ENGINEER
- THE CONTRACTOR SHALL REVIEW ALL DIMENSIONS SHOWN ON THE DESIGN/CONSTRUCTION PERMIT DRAWINGS WITH ALL THE OTHER DISCIPLINE DRAWINGS (ARCHITECTURAL, STRUCTURAL, MECHANICAL AND ELECTRICAL) AND REPORT ANY DISCREPANCIES TO THE APPLICABLE DISCIPLINE IMMEDIATELY.
- STRUCTURAL DETAILS SHALL SUPERSEDE THOSE ON TYPICAL DETAILS. IN THE CASE OF A DISCREPANCY. THE MORE STRINGENT SHALL GOVERN
- ALL WORK AND WORKMANSHIP SHALL CONFORM TO THE REQUIREMENTS OF THE ONTARIO BUILDING CODE (0.B.C.). IT IS EXPECTED THAT ALL WORK SHALL BE CARRIED OUT BY PERSONS NHO ARE KNOWLEDGEABLE AND COMPETENT WITHIN THEIR TRADES WHO ARE NOWELEDGABLE AND COMPETEUR WITHIN THEIR TRADES OF SPECIALIZATION TO CARRY OUT THE WORK AS IT PERTAINS TO THIS PROJECT. THE CONTRACTOR, IN AGREEING TO UNDERTAKE THE WORK. SHALL COMPLY WITH ALL THE DETAILED REQUIREMENTS OF THE O.B.C., SPECIFICALLY THOSE REQUIREMENTS SET FORTH IN PART
- 1.10 STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH CAN/CSA G40.2IM. WIDE FLANGE SECTIONS 350W. HSS SECTIONS ARE ASTM A500 GRADE C (FY=345MPA). ALL OTHER STEEL 300W. 1.11 ANCHOR RODS IN ACCORDANCE WITH ASTM FI554 GRADE 36
- (FY=248MPA).

 1.12 ALL STEEL CONNECTIONS SHALL BE WELDED UNLESS NOTED
- OTHERWISE, ALL WELDING IN ACCORDANCE WITH CSA W59. FLECTRODE CLASSIFICATION F49XX
- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH LATEST EDITIONS OF STANDARDS CSA A23. 1 AND CSA A23.3.
- 1.14 MINIMUM CONCRETE SPECIFIED COMPRESSIVE STRENGTH FC=25MPA
- 1.15 REINFORCING STEEL SHALL BE IN ACCORDANCE WITH CAN/CSA G30.18M GRADE 400R/400W
- 1.16 STRUCTURAL DESIGN LOADING (UNFACTORED)

GENERAL DESIGN PARAMETERS

BASIC SNOW/RAIN LOAD Ss = 1.10 kPa

LL = 1.90 kPa

Sr = 0.40 kPa

WIND LOAD FACTORS

Q (1/50)= 0.46 kPa Q (1/10)= 0.36 kPa

ROOF DL = 0.75 kPa

SL = 1.00 kPA DL = 1.00 kPa

- DESIGN WAS DONE IN ACCORDANCE WITH THE PART 9 OF THE PLAIN CONCRETE FOR FOOTINGS WHERE DESIGNED IN ACCORDANCE
- PLAIN CONCRETE FOR FUD HINGS WHERE DESIGNED IN ACCORDANCE WITH PART 9 OF THE 0.B.C. AND ALL OTHER UNREINFORCED AND REINFORCED CONCRETE WAS DESIGNED TO CSA A23.3 UNREINFORCED MASONRY FOUNDATION WALLS WHERE DESIGNED IN
- ACCORDANCE WITH PART 9 OF THE O.B.C. AND ALL OTHER UNREINFORCED AND REINFORCED MASONRY TO CSA S304.1
- STRUCTURAL STEEL DESIGN IS IN ACCORDANE CAN/CSA S16.1
- ROOF JOISTS AND CEILING JOISTS AND RAFTERS WHERE DESIGNED IN ACCORDANCE WITH PART 9 OF THE 0.B.C. AND FLOOR JOISTS AND ALL WOOD MEMBERS ARE DESIGNED TO CSA 086
- UNLESS NOTED OTHERWISE, LOADS ARE SHOWN ON THE DRAWINGS. CONSTRUCTION LOADS SHALL NOT EXCEED THOSE TABULATED IN THE DESIGN NOTES OR THE DRAWINGS
- CONTRACTOR SHALL MAKE SPECIAL PROVISION FOR THE WORK IF UNDERTAKEN IN COLD WEATHER CONDITIONS AND SHALL COMPLY WITH ALL STANDARDS OF PRACTICE PERTAINING TO COLD WEATHER CONSTRUCTION. CONTRACTOR SHALL NOTIFY THE ENGINEER IF IT IS THE INTENTION TO PERFORM WORK THROUGH COLD WEATHER CONDITIONS, PRIOR TO BEGINNING OF WORK

3. GEOTECHNICAL AND EXCAVATION WORKS

- THE LINE OF SLOPE BETWEEN ADJACENT FOOTINGS OR EXCAVATIONS OR ALONG STEPPED FOOTINGS SHALL NOT EXCEED A RISE TO RUN OF 7:10.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING ALL UTILITIES PRIOR TO EXCAVATING (I.E. "CALL BEFORE YOU DIG" ONTARIO ONE CALL).
- UNLESS NOTED ON THE DRAWINGS. THE CONTRACTOR IS REQUIRED. ONLESS NOTED ON THE DIRAWINGS, THE CONTRACT ONE REQUIRED TO VERIFY THAT THE ALLOWABLE SOIL BEARING CAPACITY IS A MINIMUM OF 3000 PSF AND PROVIDE THE DESIGNER AND/OR ENGINEER WITH A COPY OF THE GEOTECHNICAL LETTER CONFIRMING THE SOIL BEARING CAPACITY.
- CONTRACTOR SHALL PLACE FOOTINGS AND PIERS ON NATURALLY UNDISTURBED SOIL. THE EXPOSED SOIL SURFACE SHALL BE FREE FROM ALL DELETERIOUS MATERIALS. THE CONTRACTOR SHALL CONTACT THE ENGINEER IMMEDIATELY IF THEY IDENTIFY WET OR WEAK AREAS AND THESE AREAS SHOULD BE INVESTIGATED BY A GEOTECHNICAL ENGINEER.
- GEO I ECHNICAL ENGINEER.

 THE CONTRACTOR SHALL BE RESPONSIBLE FOR UNDERTAKING ALL EXCAVATION WORK AND SHALL PERFORM THE WORKS AS TO PREVENT DAMAGE TO ADJACENT STRUCTURES, PROPERTY, UTILITIES, ROADS, SIDEWALKS DURING ALL STAGES OF CONSTRUCTION.
- THE BASE AND SIDE OF EVERY EXCAVATION AREA SHALL BE FREE FROM ORGANIC MATERIAL.
- IN AREAS WHERE TERMITES ARE KNOWN TO BE PROBLEMATIC. ALL STUMPS, ROOTS, AND OTHER WOOD DEBRIS SHALL BE REMOVED FROM THE SOIL TO A DEPTH NOT LESS THAN 300mm (12') IN UNEXCAVATED AREAS UNDER A BUILDING OR STRUCTURE EXCAVATION SHALL BE FREE FROM STANDING WATER. IF THIS
- CONDITION EXISTS, THE CONTRACTOR SHALL CONTACT THE ENGINEER IMMEDIATELY. THIS CONDITION SHALL GIVE RISE TO A ENGINEER IMMEDIALELY. HIS CONDITION SHALL GIVE RISE I FURTHER GEOTECHNICAL INVESTIGATION TO CONFIRM THE ALLOWABLE BEARING CAPACITY OF THE SOIL.

 IF THE WORK IS TO PROCEED DURING WINTER MONTHS, THE
- EXCAVATED AREAS SHALL BE KEPT FROM FREEZING THROUGHOUT THE CONSTRUCTION PERIOD.

 ALL FOOTINGS AND FOUNDATIONS SHALL BE FOUNDED AT A DEPTH
- NOT LESS THAN 1.2M (4FT) BELOW GRADE, EXCEPT WHERE INSULATING MEASURES HAVE BEEN MADE TO REDUCE THE DEPTH OF FROST PENETRATION AND DIRECTION OF THE ICE LENSING

4. CONCRETE NOTES

- 4.1 DESIGN AND CONSTRUCTION OF CONCRETE SHALL CONFORM TO CSA
- 4.2 UNLESS OTHERWISE NOTED, THE CONCRETE SHALL HAVE MINIMUM
 - TYPE GU CEMENT, NOMINAL SIZE AGGREGATE OF 20mm
 - GENERAL USE CONCRETE: 25MPa AT 28 DAYS, 75mm SLUMP & 0.55
 - CONCRETE FOR EXTERIOR USE AND/OR EXPOSED TO FREEZING: 32MPa at 28 DAYS, 75mm SLUMP, 0.45 WATER/ CEMENT RATIO & 5-8% AIR ENTRAINMENT
- REINFORCED CONCRETE SHALL HAVE THE FOLLOWING COVER TO
- CONCRETE CAST AGAINST SOIL AND/OR EXPOSED TO FREEZING SHALL HAVE A 75mm (3") COVER
- CONCRETE NOT EXPOSED TO FREEZING OR CAST AGAINST SOIL SHALL HAVE A 25mm (1") COVER
- ALL ANCHOR RODS SHALL ALL BE THREADED ASTM A193 B7 ROD.
- ALL GROUT SHOWN ON THE DESIGN/CONSTRUCTION PERMIT
 DRAWINGS SHALL BE SIKAGROUT 212 OR APPROVED EQUAL. GROUT
 SHALL BE PLACED UNDER ALL COLUMN BASE PLATES TO ENSURE FULL BEARING ON THE CONCRETE.
- THE FINISHED CONCRETE PRODUCT SHALL BE PLACED IN SUCH A MANNER THAT ANY ARCHITECTURALLY EXPOSED OR COMMONLY VISIBLE CONCRETE SURFACE SHALL BE FREE FROM VISIBLE SIGNS OF STREAKING OR HONEYCOMBING.
- ALL REINFORCING STEEL SHALL BE GRADE 400MPa AND CONFORM TO
- 4.8 REINFORCEMENT SHALL BE SUPPORTED BY WIRE CHAIRS OR
- APPROVED EQUAL TO MAINTAIN CONCRETE COVER REINFORCING STEEL SHALL BE FREE FROM LOOSE SCALE, RUST, MUD, OIL OR ANY OTHER CONTAMINATE THAT MAY REDUCE THAT BOND BETWEEN THE STEEL REINFORCEMENT AND THE CONCRETE.
- 4.10 VERTICAL REINFORCEMENT IN FOUNDATION WALLS SHALL BE ONE PIECE AND NOT SPLICED. 4.11 TACK WELDING, HEATING OR CUTTING OF STEEL REINFORCEMENT IS
- PROHIBITED UNI ESS DIRECTED BY THE ENGINEER.
- 4.12 LOCATION OF FLOOR CONTROL JOINTS SHALL BE SPACED AT A MAXIMUM OF 6m (20FT) AND SHALL BE PROVIDED AROUND ALL COLUMN FOOTINGS.
- 4.13 CONCRETE SHALL CURE AS PER CSA A23 1./2
- 4.14 ALL CONCRETE SHALL BE CONSOLIDATED WITH A MECHANICAL

- THE DESIGN FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH CSA S16 AND THE CISC STANDARD CODE OF PRACTICE.

 ALL STRUCTURAL STEEL SHALL BE GRADE 350W AND CONFORM TO
- CSA G40 20-13/G40 21-13
- ALL STEEL PLATES OTHER MISCELLANEOUS SHAPES SHALL BE GRADE 300W AND CONFORM G40.20-13/G40.21-13
- WELDING PRACTICES SHALL CONFORM TO CSA W59-13.
 CONTRACTOR IS TO TAKE ALL NECESSARY PRECAUTIONS IF WELDING IS TO BE DONE ONSITE AND NEAR COMBUSTIBLE MATERIALS, WHERE CUTTING OR WELDING IS DONE NEAR WALLS, PARTITIONS, CEILING OR ROOF OF COMBUSTIBLE CONSTRUCTION, FIRE-RESISTANT SHIELDS OR GUARDS SHALL BE PROVIDED TO PREVENT IGNITION.
 CONNECTION NOT DETAILED ON THE DESIGN/CONSTRUCTION PERMIT
- DRAWINGS SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR'S ENGINEER.
- USE A MINIMUM OF 2 BOLTS FOR EVERY BOLTED CONNECTION. ALL BOLTED CONNECTIONS SHALL BE DONE USING 'TURN-ON-NU' METHOD', UNLESS NOTED OTHERWISE ON THE
- DESIGN/CONSTRUCTION PERMIT DRAWINGS ALL EXPOSED STEEL MEMBERS AND CONNECTORS SHALL BE HOT
- STEEL FABRICATOR IS RESPONSIBLE FOR THE DESIGN OF ALL STEEL STEEL FARMATOR IS RESPONSIBLE FOR THE SECOND OF A SECONDECTION S. CONNECTION SHOP DRAWINGS TO BE PREPARED BY A SPECIALTY STRUCTURAL ENGINEER RETAINED BY THE FABRICATOR AND SHALL BE SEALED AND SIGNED BY THIS ENGINEER.
- 5.10 SUBMIT FABRICATION SHOP DRAWINGS TO RWP ENGINEERING FOR REVIEW PRIOR TO START OF STEEL FARRICATION
- 5.11 THE STEEL FABRICATOR SHALL BE CERTIFIED TO CSA W47.1 DIV 1 OR 2. 5.12 ALL WELDING SHALL COMPLY WITH CSA W59.
- COLUMNS TO BE SET CENTERED ON WALLS AND FOOTINGS UNLESS NOTED OTHERWISE.
- 5.14 STRUCTURAL STEEL SHALL CONFORM TO CAN/CSA-G40-21 GRADE 300W. HOLLOW STRUCTURAL SECTION SHALL CONFORM TO CAN/CSA-G40-21 GRADE 350W CLASS 'H'.
- 5.15 REINFORCING STEEL SHALL CONFORM TO CSA-G30-18M GRADE

6. CONCRETE MASONRY (C.M.U.) NOTES

- 6.1 THE DESIGN AND ERECTION OF MASONRY ELEMENTS SHALL BE IN ACCORDANCE WITH CAN/CSA-A371-04 (R2009) - MASONRY CONSTRUCTION FROM BUILDINGS AND S304.1-04 - DESIGN OF MASONRY STRUCTURE
- CONCRETE SHALL BE TESTED AT A FREQUENCY NO LESS THAN SET OF CYLINDERS/DAY/TYPE OF CONC. OR EVERY 50 CUBIC METERS OF CONCRETE. TEST RESULTS SHALL BE PROVIDED TO THE ENGINEER.
- CONCRETE BLOCKS SHALL CONFORM TO CSA A165 AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 15MPA-H/15/D/M
- USE ONLY TYPE S MORTAR CONFORMING TO CSA-A179-04 MASONRY WALLS ARE TO BE RUNNING BOND WITH FULL MORTAR
- BEDS, UNLESS NOTED OTHERWISE. COURSING HEIGHT SHALL BE 200MM (8") FOR ONE BLOCK AND ONE JOINT. ALL MORTAR JOINTS ARE TO BE TOOLED TO A CONCAVE JOINT, BOTH
- NTERIOR AND EXTERIOR EXPOSURES. CONTINUOUS WELDED DOUBLE WIRE WELDED LADDER OR TRUSS TYPE SHALL CONFORM TO CAN/CSA-A370-04 (R2009)-CONNECTORS
- ALL MASONRY USED AS FOUNDATION WALLS SHALL BE PARGED AS PER O B C 9 15 6
- ALL REINFORCING STEEL SHALL BE GRADE 400MPA AND CONFIRM TO
- REINFORCEMENT SHALL BE SUPPORTED BY WIRE CHAIRS OR APPROVED EQUAL TO MAINTAIN CONCRETE COVER. REINFORCING STEFL BE FREE FROM LOOSE SCALE, RUST, MUD. OIL
- OR ANY OTHER CONTAMINATE THAT MAY REDUCE THE BOND BETWEEN THE STEEL REINFORCEMENT AND THE CONCRETE VERTICAL REINFORCEMENT IN FOUNDATION WALLS SHALL BE ONE
- PIECE AND NOT SLICED. 6.13 TACK WELDING, WELDING, HEATING OR CUTTING OF STEEL REINFORCEMENT IS PROHIBITED UNLESS DIRECTED BY THE ENGINEER.
- 6.14 FRECT ALL MASONRY VENEER PLUMB, SQUARE AND TRUE TO LINES 6.15 INSTALL METAL CONNECTORS AND PROPRIETARY PRODUCTS IN ACCORDANCE WITH MANUFACTURER SPECIFICATIONS AND

RECOMMENDATIONS 7. MASONRY VENEER NOTES

- DESIGN AND CONSTRUCTION OF MASONRY VENEER FOR RESIDENTIAL CONSTRUCTION SHALL CONFORM TO PART 9 OF THE O.B.C SECTIONS 9.20.64 - MASONRY VENEER, SECTION 9.20.9.5- TIES FOR
- MASONRY VENEER SHALL BE SOLID WITH A MINIMUM COMPRESSIVE STRENGTH OF 15MPa
- MASONRY VENEER SHALL BE LAID IN A RUNNING BOND PATTERN.
- MORTAR AND GROUT SHALL CONFORM TO CSA-A173-04. USE ONLY TYPE S MORTAR FOR ALL CONSTRUCTION. ALL MORTAR JOINTS ARE TO BE TOOLED TO A CONCAVE JOINT, BOTH INTERIOR AND EXTERIOR EXPOSURES.
- MASONRY VENEER TIES SHALL HAVE A MAXIMUM VERTICAL SPACING 0F 400MM (16") AND A MAXIMUM HORIZONTAL SPACING OF 400MM (16"). THE VERTICAL SPACING SHALL MATCH EVERY VERTICAL STUD
- TIES SHALL BE CORROSION-RESISTANT METAL TIES NAILED TO THE STUDS AND EMBEDDED IN THE MORTAR JOINTS BETWEEN THE MASONRY TO TIE THE VENEER TO THE FRAMEWORK.

 MASONRY TIES SHALL NOT BE LESS THAN 0.76MM THICK AND 22MM
- WIDE, CORROSION RESISTANT AND SHAPED TO PROVIDE A KEY WITH THE MORTAR JOINT, MASONRY STRAPS ARE NOT PERMITTED.
- MASONRY VENEER SHALL NOT PROJECT MORE THAN 30MM BEYOND THE FACE OF THE SUPPORTING BASE, PROVIDED THAT THE UNITS ARE AT LEAST 90MM (3 1/2") THICK.
- 7.10 ALL MASONRY VENEER LINTELS SHALL BE HOT DIPPED GALVANIZED. 7.11 ALL MASONRY VENEER LINTELS SHALL SUPPORT AT LEAST TWO THIRDS OF THE VENEER THICKNESS.
- 7.12 ERECT ALL MASONRY VENEER PLUMB, SQUARE AND TRUE TO LINES. 7.13 INSTALL METAL CONNECTORS AND PROPRIETARY PRODUCTS IN ACCORDANCE WITH MANUFACTURER SPECIFICATIONS AND

- 8.1 DESIGN AND CONSTRUCTION OF WOOD MEMBERS AND CONNECTORS SHALL CONFORM TO PART 9 OD THE O.B.C. CSA 086 & CWO "ENGINEERING GUIDE FOR WOOD FRAME CONSTRUCTION"
- ALL WOOD EXPOSED TO THE EXTERIOR SHALL BE PRESERVATIVE TREATED. EXTERIOR PLYWOOD SHEATHING SHALL BE STAMPED EXTERIOR GRADE, SHEATHING SHALL CONFORM TO CSA 0151 AND BE GRADE D-FIR PLYWOOD. OSB BOARD IS NOT PERMITTED ON ANY EXTERIOR SURFACE.
- SAWN LUMBER SHALL CONFORM TO CSA 0141 AND BE STAMPED SPF NO. 2 OR GREATER
- IN AREAS WHERE TERMITES ARE KNOWN TO OCCUR, DESIGN AND CONSTRICTION SHALL CONFORM TO CLAUSE 9.3.2.8 OF THE O.B.C
- 8.5 FRECT ALL WOOD FRAMING PLUMB, SQUARE AND TRUE TO LINES.
- COMMON WIRE NAILS SHALL PENETRATE THE WOOD SUBSTRATE PER THE FOLLOWING TABLE

SIZE	DIAMETER	WIRE GAUGE	PENETRATION
8d	3.3mm (0.131")	10.25	38mm (1.5")
10d	3.8mm (0.148 ")	9	41mm (1.625")
16d	4.1mm (0.1625")	8	45mm (1.75")
20d	4.9mm (0.192")	6	54mm (2.125")

NOTE: PENETRATION IS MEASURED INTO THE PIECE OF WOOD RECEIVING THE NAIL. 38mm (1.5") OF PENETRATION IS ACCEPTABLE FOR 10D AND 16D NAILS FOR TOP PLATE SAND 38mmx (2x) MEMBERS.

- 8.7 STEEL WIRE NAILS OR COMMON SPIRAL NAILS. SPIKES, AND STAPLES SHALL CONFORM TO ASTM F 1667 OR CSA B111. ALL FASTENERS SHALL BE HOT DIPPED GALVANIZED IF USED FOR EXTERIOR
- 8.8 HOLES SHALL BE DRILLED TO PREVENT SPLITTING OF WOOD AS
- 8.9 INSTALL ENGINEERED LUMBER, METAL CONNECTORS AND PROPRIETARY PRODUCTS IN ACCORDANCE WITH MANUFACTURER SPECIFICATIONS AND RECOMMENDATIONS.
- TOP PLATES SHALL BE CONSTRUCTED OF TWO PLATES, SAME SIZE AS STUD, STAGGERED SPLICES MINIMUM OF 1220MM (4-0*). CENTRE SPLICES OVER STUDS. SPLICES SHALL CONSTRUCTED WITH A MINIMUM OF 1-16D NAILS.
- BUILT -UP WOOD MEMBER SHALL CONFORM TO CSA 086 CLAUSE 5.5.6.4 AND BE NAILED TOGETHER WITH (2)-75MM LONG NAILS EVERY 200MM ON CENTERS AND WITHIN 60MM FROM EACH END. HOT DIPPED GALVANIZED NAILS SHALL BE USED IF EXPOSED TO THE ELEMENTS.
- SOLID BLOCKING OR CROSS BRACING SHALL BE INSTALLED FOR ALL FLOOR JOISTS. BLOCKING/BRACING SHALL BE PROVIDED WITHIN 2.11 (6-10") FROM EACH SUPPORT AND THE SPACING OF BLOCKING/BRACING SHALL BC SPACING OF BLOCKING/BRACING SHALL NOT EXCEED 2.1M (6-10").
- 8 13 ALL OPENINGS SHALL BE REINFORCED WITH A MINIMUM OF DOLIBLE HEADERS AND DOUBLE TRIMMERS, UNLESS NOTED OTHERWISE ON THE STRUCTURAL PLANS. 8.14 NON-LOAD BEARING WALLS SHALL HAVE DOUBLE JOISTS PROVIDED
- WHEN THE JOISTS RUN PARALLEL WITH THE WALL OR SOLID BLOCKING SHALL BE PROVIDED WHEN THE JOIST ARE PERPENDICULAR TO THE WALL. 8.15 END SUPPORTS OF ALL ROOF AND JOISTS SHALL HAVE THEIR ENDS HELD IN POSITION BY EITHER; SOLID BLOCKING, NAILED BRIDGING
- NAILING TO OTHER MEMBERS OR JOISTS HANGERS.

 8.16 ALL FLOOR AND ROOF SHEATHING SHALL HAVE A MINIMUM OF THICKNESS OF 19MM (3/4"), T&G, GLUED AND NAILED TO FLOOP
- WALL SHEATHING SHALL HAVE A MINIMUM OF THICKNESS PF 127MM (1/2") AND BE OF A PLYWOOD CONSTRUCTION.
- 8.18 ALL WALLS OVER 244M (8'-0") HIGH SHALL HAVE BLOCKING PROVIDED
- AT MID-HEIGHT OF THE STUDS.

 8.19 ALL WOODS PRODUCTS SHALL BE KEPT FROM THE GROUND AND SHALL BE PROTECTED FROM THE EXTERIOR ENVIRONMENT
- 8.20 THE STRUCTURE SHALL NOT BE FULLY ENCLOSED UNTIL THE WOOD MOISTURE CONTENT HAS BEEN VERIFIED TO BE AT OR BELOW 15% ANY SIGNS OF MOLD OR ROT SHALL BE REMOVED IMMEDIATELY AND REPLACED BY AN ACCEPTABLE WOOD ELEMENT.
- 8.21 WHERE ERAMING HANGERS ARE REQUIRED BUT HAVE NOT BEEN WHERE FRAMINIO FAINCERS ARE REQUIRED BUT HAVE NOT BEEN SPECIFIED BY THE ENGINEER, THE CONTRACTOR SHALL CONTACT THE ENGINEER TO OBTAIN THE APPROPRIATE WOOD CONNECTOR BEFORE PROCEEDING WITH THE WORK. 8.22 NOTCHING OR DRILLING HOLES IN FLOOR JOISTS OR WALL STUDS IS
- STRICTLY PROHIBITED. THE CONTRACTOR SHALL OBTAIN CONSENT FROM THE ENGINEER BEFORE NOTCHING OR DRILLING HOLES. 8.23 TIMBER FRAMING DESIGNED TO THE LATEST STANDARD CSA 086
- 8.24 ENSURE MINIMUM BEARING LENGTH OF 1-1/2" FOR JOISTS AND 3" FOR BEAMS UNLESS NOTED OTHERWISE. 8.25 ALL BUILT-UP WOOD COLUMNS AND BUILT-UP WOOD BEAMS TO BE
- SPLICED AS PER OBC PART 9. PROVIDE ADEQUATE RESTRAINT TO THE TOPS AND BOTTOMS OF ALL
- 8.27 REFER TO PLANS FOR STRUCTURAL PANEL SHEATHING NAIL 8.28 ALL NAILS/SCREWS/BOLTS EXPOSED TO THE ELEMENTS TO BE
- HOT-DIPPED GALVANIZED, STAINLESS STEEL, OR COATED WITH EQUIVALENT CORROSION PROTECTION COATING AS PER MANUFACTURES 8.29 SIMPSON STRONG TIE CONNECTORS TO BE PROVIDED AS INDICTED IN
- THE DRAWINGS, ALL CONNECTORS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS 8.30 ENGINEERED I-JOISTS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

SHOP DRAWINGS REVIEW:

REVIEW OF SHOP DRAWINGS IS FOR GENERAL CONFORMITY WITH STRUCTURAL CONTRACT DOCUMENTS AND SPECIFICATIONS ONLY, ANY COMMENTS MADE ON THE SUBMITTED SHOP DRAWINGS DO NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE REQUIREMENT OF THE STRUCTURAL CONTRACT DOCUMENTS AND SPECIFICATIONS NOR DO THEY AUTHORIZE CHANGES TO THE CONTRACT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL QUANTITIES. FIELD MEASUREMENTS. DETAIL DIMENSIONS. COORDINATION WITH ALL TRADES. FABRICATION PROCESSES AND PROCEDURES. METHODS, MEANS, AND SEQUENCE OF CONSTRUCTION AND THE PERFORMANCE OF ALL WORK IN ACCORDANCE WITH STANDARD INDUSTRY PRACTICE IN A SAFE MANNER. THE REVIEW OF SHOP DRAWINGS DOES NOT IMPLY ANY CHANGE TO NOR DIMINISH THE RESPONSIBILITIES OF OTHER CONSULTANTS IN THE CARRYING OUT OF THEIR WORK

THE WITH OF THE BEAM IT SUPPORTS)			
L1	STEEL LI	NTEL:	
L1	LINTEL	SIZE	
L2			mm)
L3			
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NOTE: SOLID LOAD BEARING (THE WIDTH OF THE STUD POST SHALL NOT BE LESS THAN THE WITH OF THE BEAM IT SUPPORTS)			
ALL SOLID LOAD BEARING POINTS MUST BE CONTINUOUS	NOTE: SO (THE WIE THE WIT	DLID LOAD BEARING OTH OF THE STUD POST SHALL NOT BE H OF THE BEAM IT SUPPORTS)	

AND CARRIED DOWN TO BEAMS, FOUNDATION WALLS OR FOOTINGS. PROVIDE BLOCKING AS REQUIRED

- PROVIDE THE FOLLOWING BEARING LENGTH
- 1-I" FOR ENGINEERING JOISTS 3-" FOR LVL AND WOOD BEAMS - 6" FOR STEEL LINTELS AND STEEL BEAMS BACK-TO-BACK STEEL LINTELS SHALL BE BOLTED

TOGETHER W/4" DIA A307 BOLTS (W/ NUTS AND

WASHERS) @ 12" o.c. OR WELDED W/ 4" FILLET WELDS

FOLLOW MANUFACTURES SPECIFICATIONS FOR ALL

FGFND:

SB

CANT.

DB.JST DOUBLE JOIST - POINT LOAD ABOVE

SUPPORTSI

B.E.W. - BOTTOM FACH WAY SBFA - SOLID BEARING FROM ABOVE

SA SMOKE ALARM CMD - CARBON MONOXIDE DETECTOR

SOLID LOAD BEARING: (THE WIDTH

OF THE STUD POST SHALL NOT BE

FD - FLOOR DRAIN

R.O. - ROUGH OPENING C.O. - CONCRETE OPENING

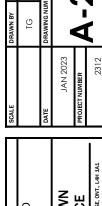
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РΤ - PRESSURE TREATED WOOD

NOTE: CONTRACTOR TO VERIFY ALL EXISTING STRUCTURAL CONDITIONS & NOTIFY DESIGNER 8 ENGINEER OF ANY DISCREPANCIES PRIOR AND DURING CONSTRUCTION.

NOTE: THE CONTRACTOR SHALL PROVIDE AND IS RESPONSIBLE FOR ALL TEMPORARY SUPPORT. SHORING, OR BRACING AS REQUIRED TO COMPLETE THE WORK

NOTE: TONED WALL REPRESENT EXISTING WALLS TO REMAIN (TYP.)

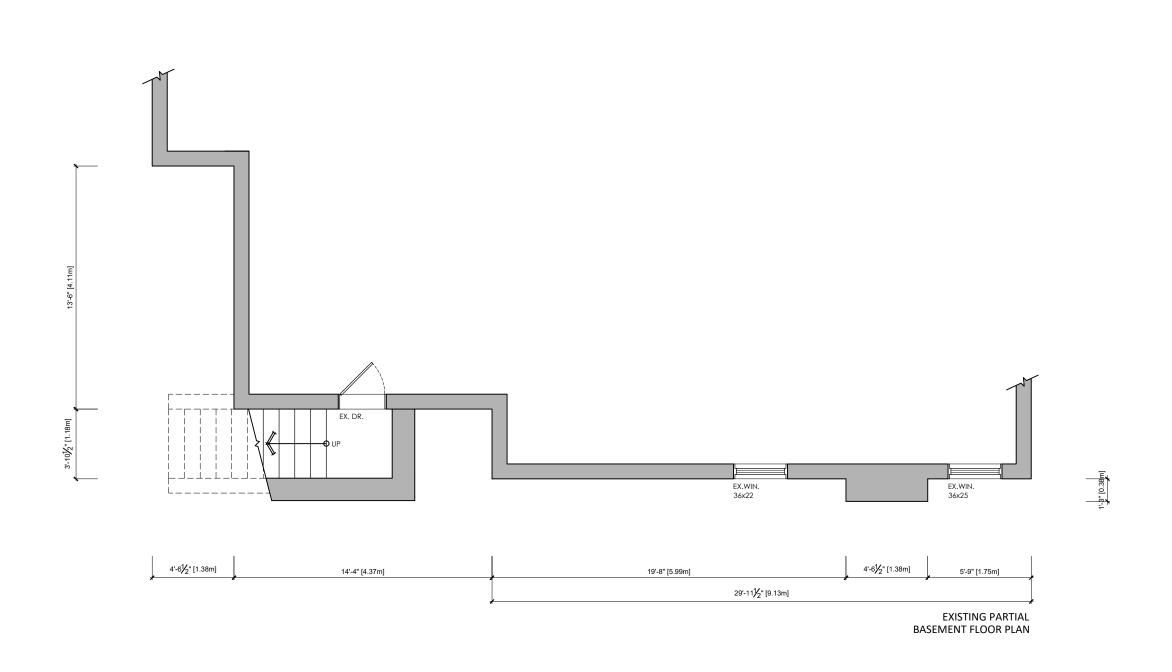






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3/16"-1"-0" TG

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JAN 2023
PROJECT NUMBER
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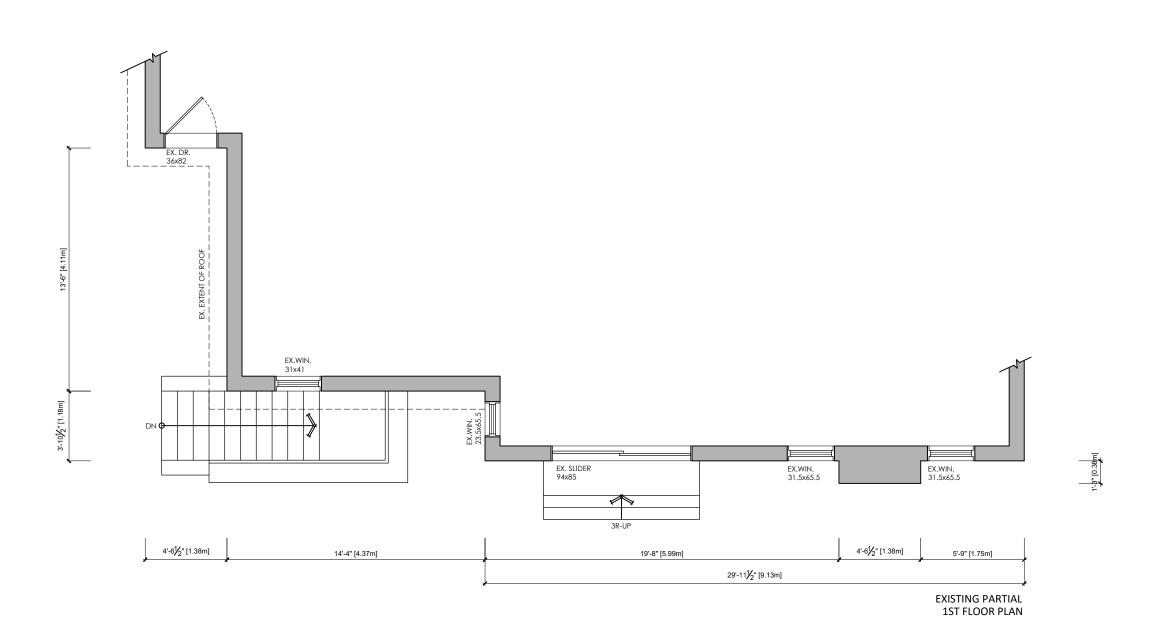
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3/16" - 1'-0" TG

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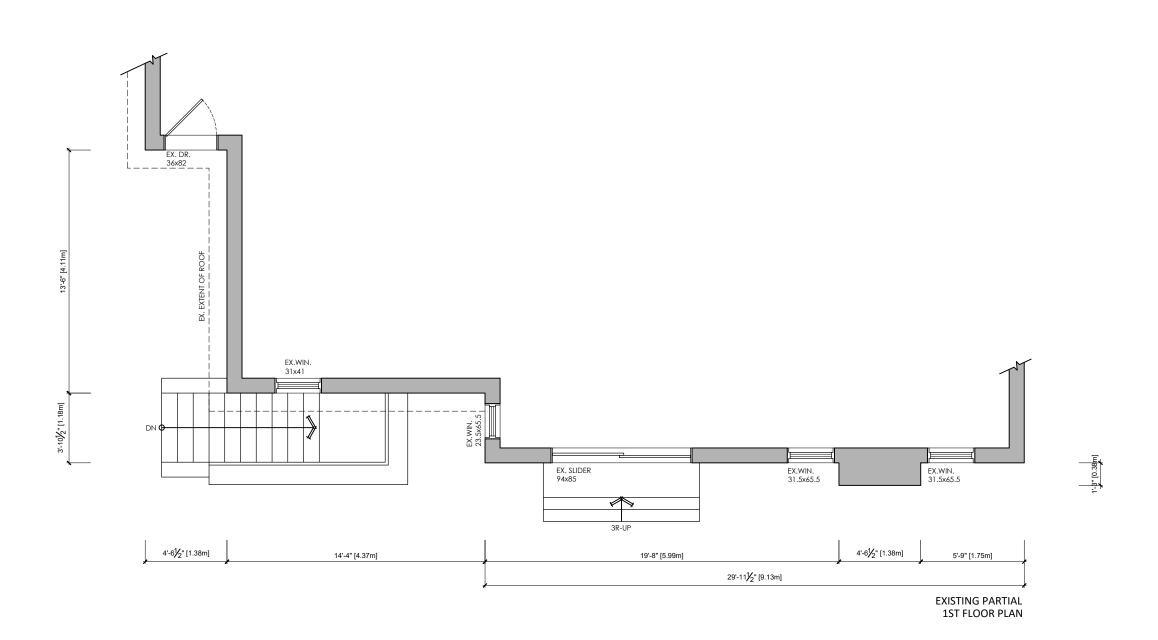
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3/16" - 1"-0" TG

DATE

JAN 2023

PROJECT NUMBER

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EXISTING PARTIAL FLOOR PLAN	SOUTHLAWN RESIDENCE	17 SOUTHLAWN DRIVE, WOODBRIDGE, ONT., L4H 1A1
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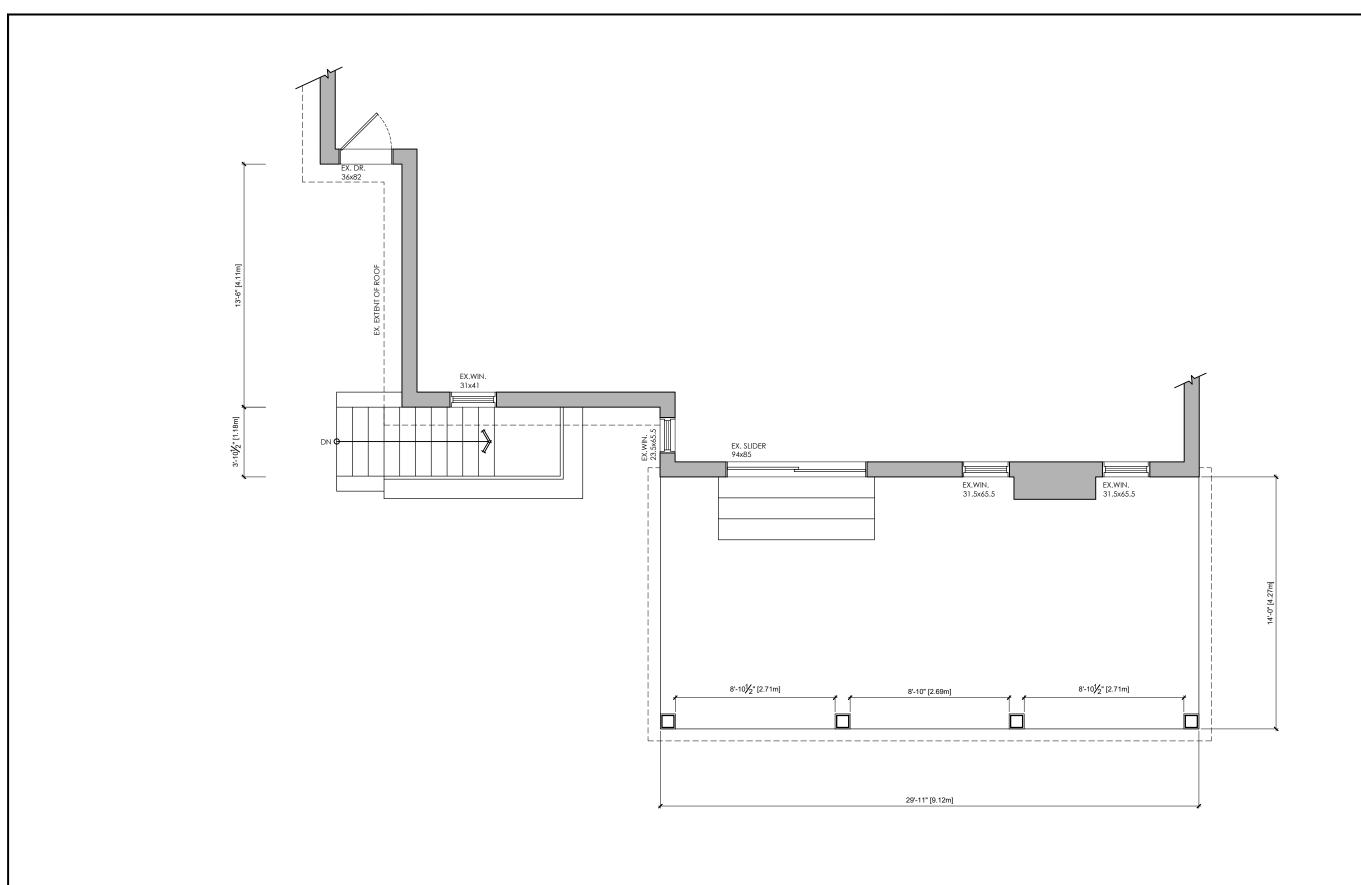




EXISTING SOUTH (REAR) ELEVATION

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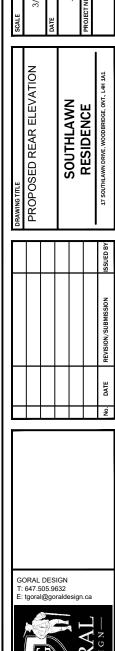


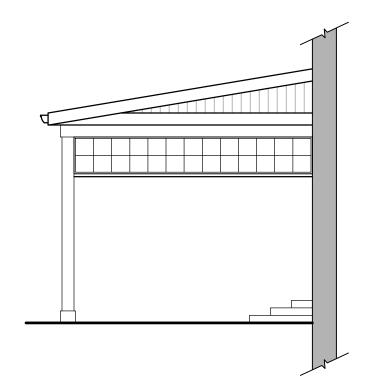
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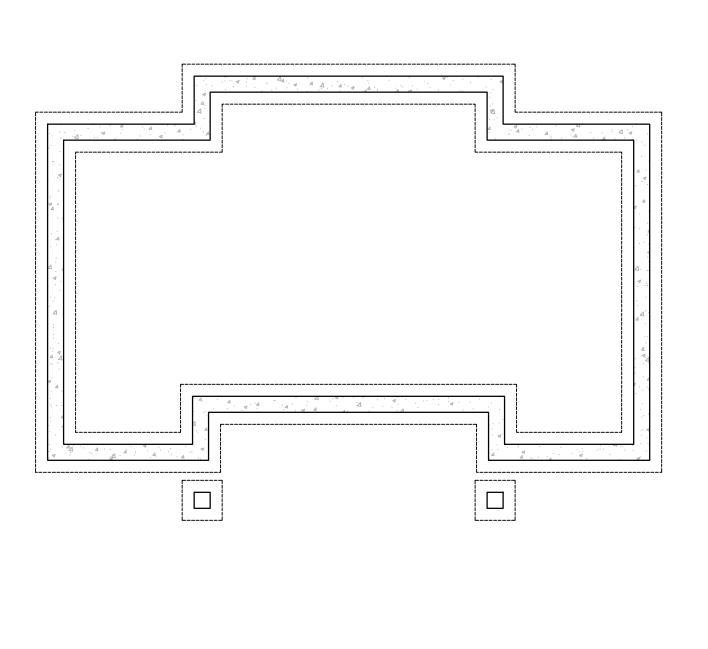






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RESIDENCE
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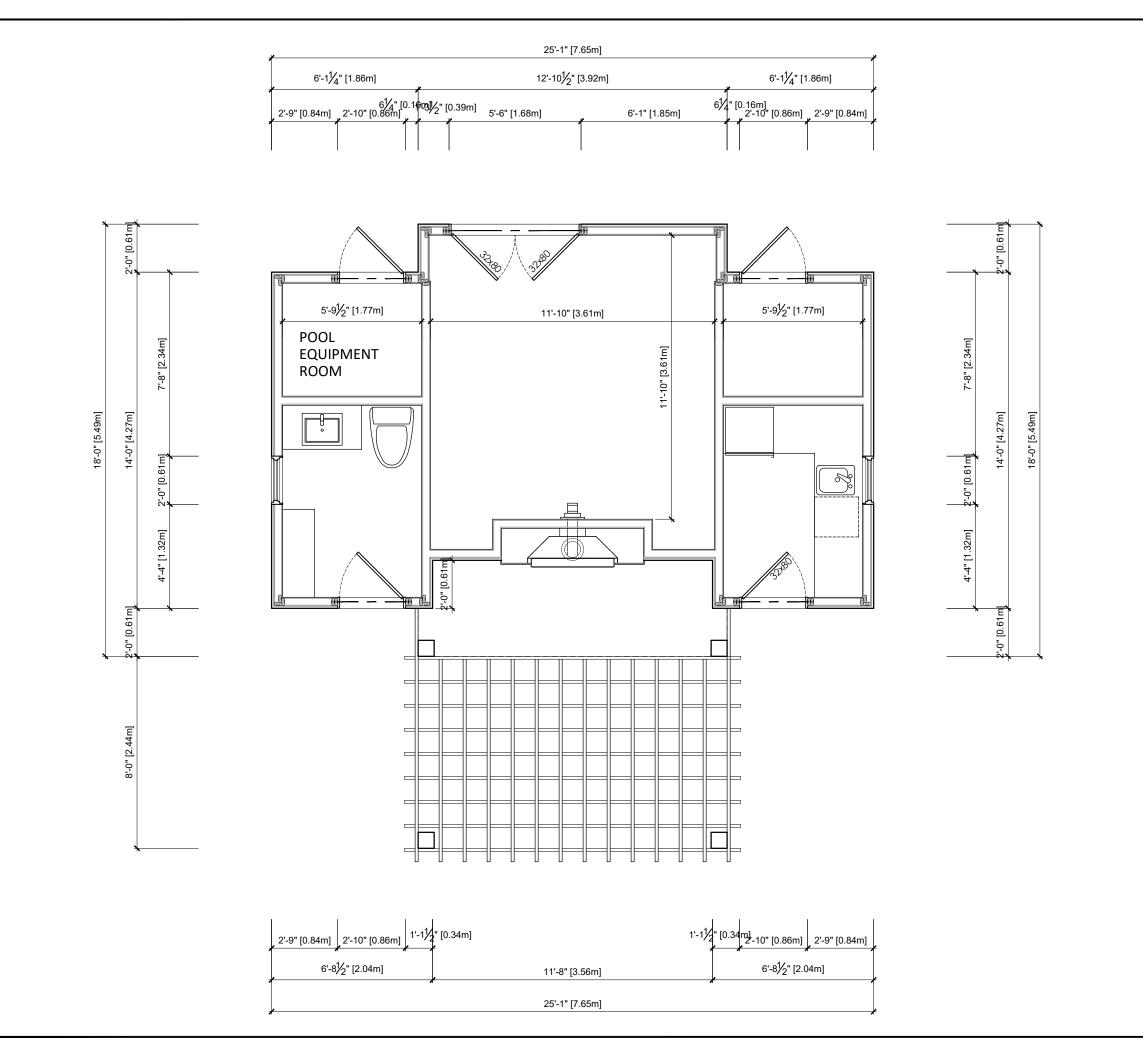
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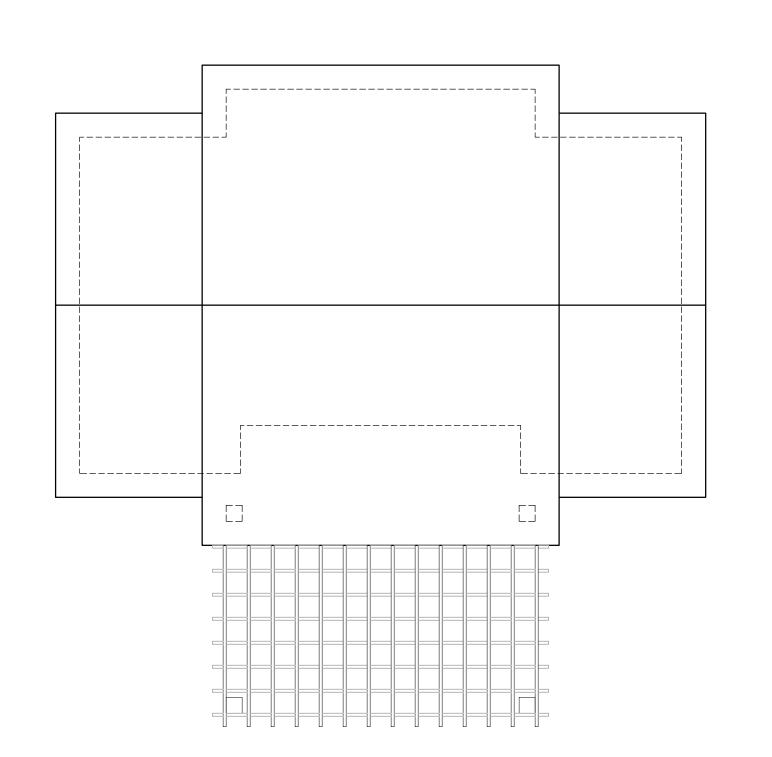
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SCALE

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PLAN

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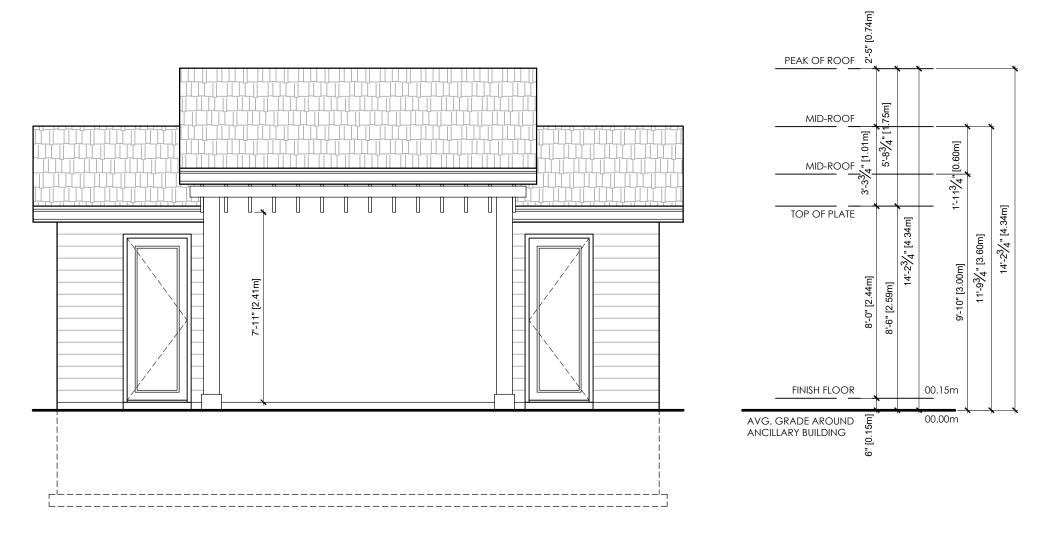
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FRONT EAST ELEVATION

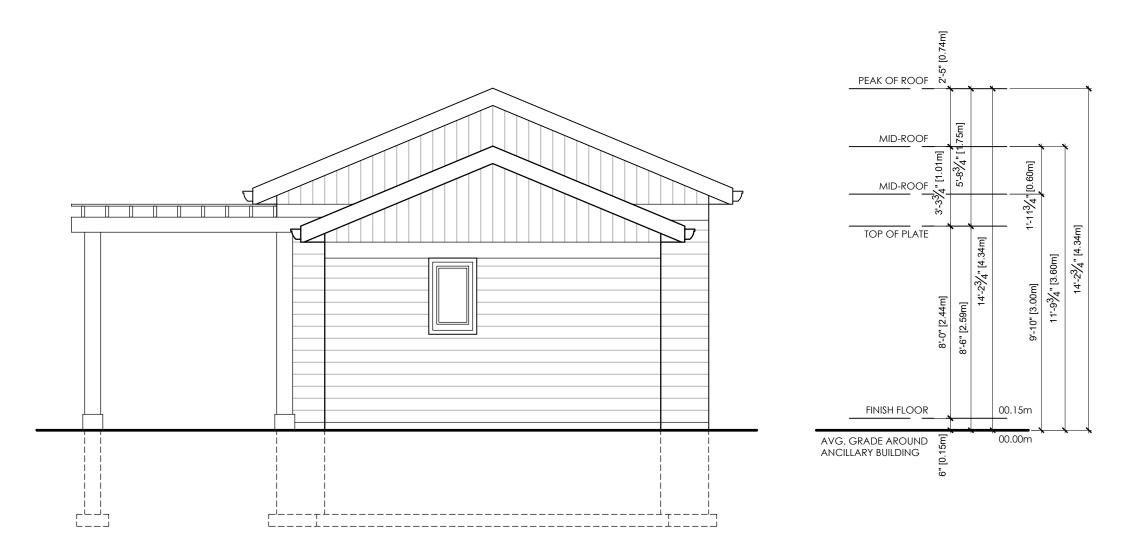
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NORTH SIDE ELEVATION

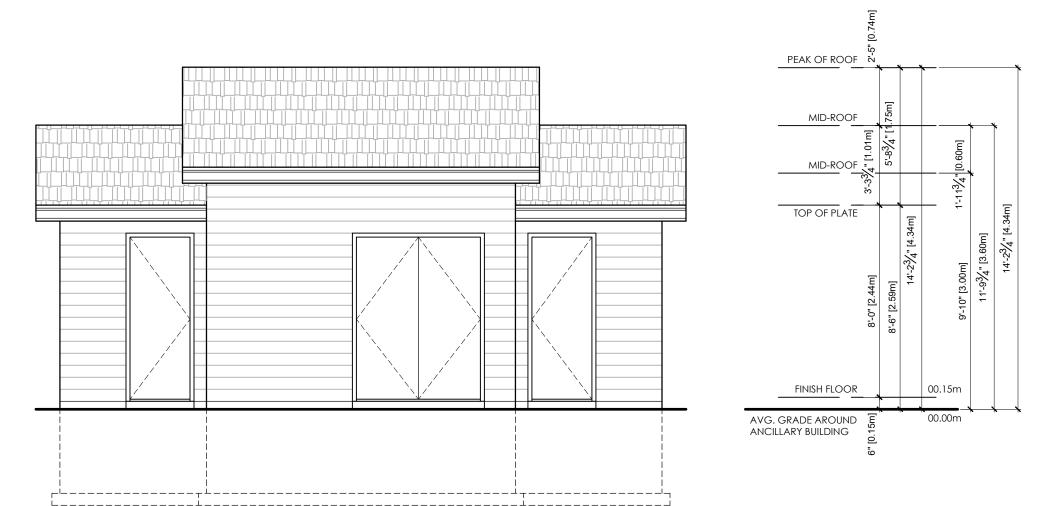
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FRONT EAST ELEVATION

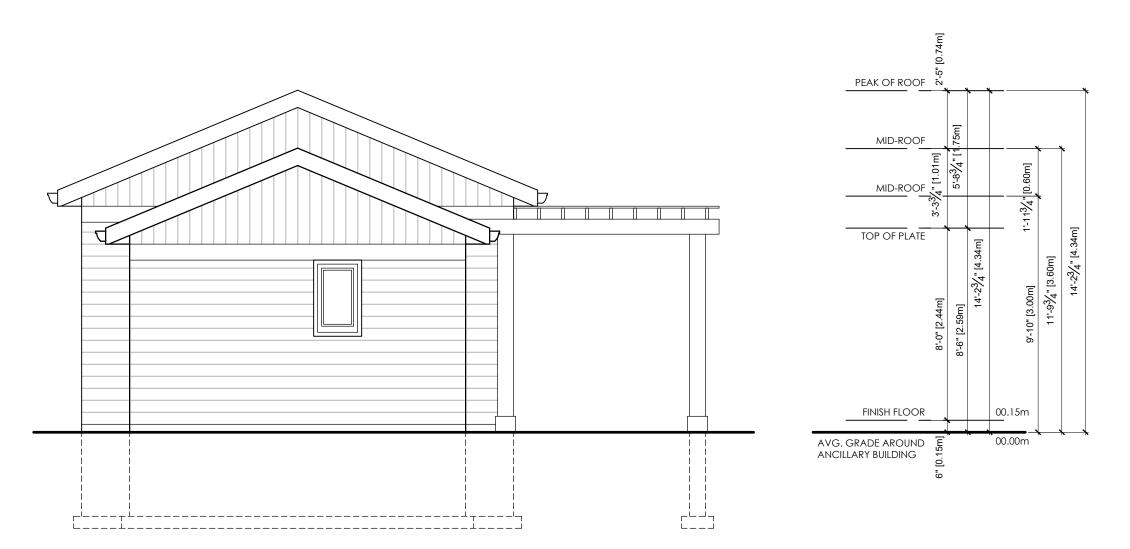
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